Infrastructure Committee Committee Agenda

Date:	Thursday, 10 May, 2018
Time:	10:30 am
Location:	Council Chamber Forum North, Rust Avenue Whangarei
Elected Members:	Her Worship the Mayor Sheryl Mai (Chairperson) Cr Stu Bell Cr Gavin Benney Cr Crichton Christie Cr Vince Cocurullo Cr Tricia Cutforth Cr Shelley Deeming Cr Sue Glen Cr Phil Halse Cr Cherry Hermon Cr Greg Innes Cr Greg Martin Cr Sharon Morgan Cr Anna Murphy

For any queries regarding this meeting please contact
the Whangarei District Council on (09) 430-4200.

Infrastructure Committee – Terms of Reference

Membership

Chairperson: Councillor Greg Martin

Members: Her Worship the Mayor Sheryl Mai
Councillors Stu Bell, Gavin Benney, Crichton Christie, Vince Cocurullo, Tricia Cutforth, Shelley Deeming, Sue Glen, Phil Halse, Cherry Hermon, Greg Innes, Sharon Morgan, Anna Murphy

Meetings: Monthly

Quorum: **7**

Purpose

To oversee the management of council's infrastructural assets, utility services and public facilities.

Key responsibilities include:

- Services including the provision and maintenance of:
  - Infrastructure projects and support
  - Infrastructure project co ordination
  - Transportation
  - Waste and Drainage
  - Water
  - Parks and Reserves.
- Shared Services – investigate opportunities for Shared Services for recommendation to council.

Delegations

(i) All powers necessary to perform the committee's responsibilities, including, but not limited to:

(a) the approval of expenditure of less than $10 million plus GST.

(b) approval of a submission to an external body.

(c) establishment of working parties or steering groups.

(d) power to establish subcommittees and to delegate their powers to that subcommittee.

(e) the power to adopt the Special Consultative Procedure provided for in Section 83 to 88 of the LGA in respect of matters under its jurisdiction (this allows for setting of fees and bylaw making processes up to but not including adoption).

(f) the power to delegate any of its powers to any joint committee established for any relevant purpose under clause 32, Schedule 7 of the Local Government Act 2002

Item 3

Infrastructure Committee Meeting Minutes

Date:	**Thursday, 12 April, 2018**
Time:	**10:30 a.m.**
Location:	**Council Chamber** **Forum North, Rust Avenue** **Whangarei**
In Attendance	**Cr Greg Martin (Chairperson)** **Cr Stu Bell** **Cr Crichton Christie** **Cr Tricia Cutforth** **Cr Shelley Deeming** **Cr Sue Glen** **Cr Phil Halse** **Cr Cherry Hermon** **Cr Greg Innes** **Cr Sharon Morgan** **Cr Anna Murphy**
Not in Attendance	**Her Worship the Mayor Sheryl Mai** **Cr Gavin Benney** **Cr Vince Cocurullo**
Scribe	**C Brindle (Senior Democracy Adviser)**

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1. Declarations of Interest

2. Apologies

Crs Vince Cocorullo and Gavin Benney (leave of absence)
Her Worship the Mayor (absent)

Moved By Cr Crichton Christie
Seconded By Cr Sue Glen

That the apologies be sustained

Carried

3. Confirmation of Minutes of Previous Infrastructure Committee Meeting

3.1 Minutes Infrastructure Committee 8 March 2018

Moved By Cr Greg Innes
Seconded By Cr Sue Glen

That the minutes of the Infrastructure Committee meeting held on Thursday 8 March 2018, including the confidential section, having been circulated, be taken as read and now confirmed and adopted as a true and correct record of proceedings of that meeting.

Carried

4. Decision Reports

4.1 Waitaua Awa Restoration Project – He Kakano Community Nursery Upgrade

Moved By Cr Greg Innes
Seconded By Cr Anna Murphy

That the Infrastructure Committee approves the funding application of $18,000 to enable the upgrade of facilities at the He Kakano Community Nursery.

Carried

4.2 Solid Waste Services - Project Control Group

Moved By Cr Greg Innes
Seconded By Cr Stu Bell

That the Committee appoints the following two additional elected members to participate in the recycling services project control group:

- Councillor Tricia Cutforth
- Councillor Anna Murphy.

Carried

4.3 Approve Community Garden Policy

Moved By Cr Anna Murphy
Seconded By Cr Sue Glen

That the Infrastructure Committee approves the Community Garden Policy located at attachment 1; subject to to the General Conditions of Occupation clause being amended to 'Permission will be granted for ~~five~~ three years.'

Carried

4.4 CON14032 Water Reticulation Maintenance Contract Extension 1

Moved By Cr Shelley Deeming
Seconded By Cr Sharon Morgan

That the Infrastructure Committee approves;

1. an extension of CON14032 Water Reticulation Maintenance to Downer NZ Ltd, from 1 July 2018 until 30 June 2020.

2. that the contract value be increased to $9,783,218 (excluding GST).

Carried

4.5 Temporary road closure - ANZAC Dawn Parade

Moved By Cr Phil Halse
Seconded By Cr Shelley Deeming

That the Infrastructure Committee

1. Approves the following roads to temporarily be closed to ordinary traffic for the ANZAC Dawn Parade in accordance with the Local Government Act (1974)

    a. **Wednesday 25th April 2018**

    Bank Street from Hunt Street to Water Street
    Rust Avenue from Whangarei Intermediate School to Bank Street
    Cameron Street from Bank Street to Rathbone Street
    Rathbone Street from Cameron Street to Robert Street
    Robert Street from Rathbone Street to Laurie Hall Carpark
    Laurie Hall Carpark
    Service Lanes adjacent to the closed roads

    Period of Closure 4:00am – 8:00am

2. Approves the side roads off the roads to be closed also be temporarily closed for a distance of up to 100 metres from the intersection for safety purposes.

3. Delegates the Chair of the Infrastructure Committee and the Infrastructure Services Group Manager the power to consider

objections and cancel or amend any or all of the temporary road closures if applicable.

Carried

5. Information Reports

5.1 Contracts Approved Under Delegated Authority April 2018

Moved By Cr Shelley Deeming
Seconded By Cr Sharon Morgan

That the Infrastructure Committee note the Infrastructure contracts awarded under Chief Executive and General Manager delegated authority.

Carried

Crs Bell, Christie and Halse requested their votes against be recorded.

5.2 Councils involvement in greenwaste management

Moved By Cr Cherry Hermon
Seconded By Cr Anna Murphy

That the Committee notes the report on Green Waste Management.

Carried

5.3 Northland Transportation Alliance Quarterly Report

Moved By Cr Stu Bell
Seconded By Cr Sharon Morgan

That the Committee

1. note the report *Northland Transportation Alliance Quarterly Report,* by the Northland Transportation Alliance Manager, Peter Thomson, dated 15 March 2018 (located at Attachment 1).

Carried

Cr Murphy left the meeting at 11.50am during discussions on Item 5.3.

5.4 Infrastructure Operations Report - April 2018

Moved By Cr Sharon Morgan
Seconded By Cr Sue Glen

That the Infrastructure Committee notes the Infrastructure Operations Report April 2018 update.

Carried

6. Public Excluded Business

There was no business conducted in public excluded.

Acknowledgement of Award

At the recent Cemeteries and Crematoria Conference 2018, Hayden Parr, Senior Cemetery Operator, was awarded the individual excellence award which recognises excellence within the cemeteries and crematoria industry.

Formal acknowledgement will be sent from the Council.

7. Closure of Meeting

The meeting concluded at 12.04pm

Confirmed this 10th day of May 2018

Councillor Greg Martin (Chairperson)

4.1 Land Development Stabilisation – Policy and Technical Design Requirements

Meeting: Infrastructure Committee

Date of meeting: May 10 2018

Reporting officer: Matthew de Boer

1 Purpose

To adopt the land development stabilisation policy and technical standard.

2 Recommendations

That the Infrastructure Committee adopts;

1. the Land Development Stabilisation - Policy
2. the Land Development Stabilisation – Technical Design Requirements.

3 Background

The geology of Whangarei District is such that unstable parcels of land often require engineered drainage interventions to render the land developable. The need for long-term maintenance on some underground drainage solutions can increase risk of failure in the medium to long term. Council engineers consistently advised caution when approving subsurface drainage systems, and advocated against the vesting of such structures to Council.

The need for a Council policy defining technical requirements for land stabilisation systems was identified in 2010, following the mapping of geotechnical hazard zones by Tonkin & Taylor and their subsequent inclusion in the District Plan. In 2011, following advice from Tonkin & Taylor, Council adopted the Subsoil Drains Policy, which outlined Council's position on the use of subsurface drainage solutions for land stabilisation.

Tonkin and Taylor were again engaged to review the technical aspects of the policy in 2016. Their report was subsequently peer-reviewed by CMW Geosciences. WDC staff held an internal workshop to review the report and identify potential risks. Subsequently, the draft Stabilisation Systems for Land Development policy and technical standard was developed.

These documents were reviewed by Tonkin and Taylor in 2016, providing detailed comments and recommendations. Tonkin &Taylor drafted new Technical Design Requirements as lead authors, with reference to applicable professional standards, guidelines, procedures and practices.

The draft documents were then distributed to the Geotechnical engineering community, with feedback received via written responses and a practitioner's workshop held at Council in 2017. Feedback was assessed and incorporated where necessary into the documents during subsequent reviews by Tonkin & Taylor, resulting in the final policy documents.

4 Discussion

The purpose of the Policy is to protect ratepayers from future liabilities. It provides clarity on non-technical requirements such as asset ownership and location, and aims to provide a readable document that clearly states Council's position. Technical requirements are covered by the Technical Design Requirements document, which outlines acceptable engineering solutions and defines requirements for different levels of slope instability.

As part of the review, Tonkin & Taylor have assessed what they consider an acceptable risk with regard to use of subsoil drainage as the sole stabilisation system. They have requested Council confirm that they are comfortable with this level of risk.

Specifically, Tonkin & Taylor suggest that Council *"review the AGS(2007c) guideline description of "Moderate Risk" and confirm applying Case 3 design measures are consistent with WDC's level of tolerable risk"* (Section 2.3.2 and 2.3.3). The document identifies suitable measures to mitigate risks in Section 2.3.3 (a-j).

'Case 3' refers to the consideration of solutions using only subsurface drainage, and applies to sites considered '*very low to moderate risk*' of landslide damage only. Qualitative descriptions of risk to property are described in Appendix C of the Australian Geomechanics Society (2007c) report (Attachment C, p91-92).

The proposal by Tonkin & Taylor include acceptance of subsoil drains as the sole means of stabilisation in very low to moderate risk situations. It is the staff recommendation that this approach is adopted. The documents have been drafted on this basis.

4.1 Policy and planning implications

Implications of approving the policy and technical design requirements
Council has a consistent benchmark by which to assess land development stabilisation systems, in line with other Councils.

There may be an impact on land values for property relying on subsurface stabilisation methods as the only economically feasible method for development.

Implications of NOT approving the policy and technical design requirements
Council will assess land development stabilisation applications on a case-by-case basis, without a clear policy direction, design guidelines or risk assessment criteria. This could potentially result in protracted negotiations with developers.

Further implications: a) Council may inherit sub-surface drainage assets with associated maintenance and landslide risks; b) Council may decline development applications if conditions appear to pose intolerable levels of risk.'

The Policy and Technical Standard will be referenced as part of the revised Environmental Engineering Standards.

4.2 Options

1. Adopt the policy and technical standard.

2. Adopt the policy with revision, such as a change to the acceptable level of risk for use of subsoil drains as the sole means of stabilisation in very low to moderate risk situations.

3. Do not adopt the policy and technical standard.

4.3 Risks

The Policy and Technical Design Requirements have been developed to reduce Council's level of, and exposure to risk for land development stabilisation using subsurface drainage solutions.

5 Significance and engagement

The decisions or matters of this Agenda do not trigger the significance criteria outlined in Council's Significance and Engagement Policy, and the public will be informed via Agenda publication on the website.

6 Attachments

1. Land Development Stabilisation – Policy (WDC, 2018).
2. Land Development Stabilisation – Technical Design Requirements (Tonkin & Taylor Ltd. 2018).
3. Practice Note guidelines for landslide risk management [Journal]. - [s.l.] : Australian Geomechnics, 2007. - 1 : Vol. 42. (Australian Geomechanics Society 2007c).

Whangarei District Council

Land Development Stabilisation – Policy FINAL.docx

Policy #0129

April 2018

Land Development Stabilisation – Policy FINAL.docx			
Audience (Primary)	External	**Business Owner (Dept)**	Waste & Drainage
Policy Author	Thane Richardt	**Next Review date**	December 2021

1. Purpose

Whangarei District Council (Council) has been receiving requests from developers to accept, and in some cases vest in Council's ownership, engineered systems designed to improve the stability and suitability of land. The development of such unstable or marginally stable land requires careful consideration to avoid future liabilities for Council and the community.

The intent of this document is to formalise Council's Policy on the use of stabilisation systems for land development.

The technical content of this Policy has been reviewed for WDC by Tonkin & Taylor Ltd.

This Policy supersedes Council's *Subsoil Drains Policy* dated October 2011.

2. Glossary of Terms

TERM	DEFINITION
Building	As defined by the New Zealand Building Code.
Consent notice	A consent notice is a form of covenant between the **Council** and a land owner and can only be imposed through a development consent (i.e. when some kind of subdivision or boundary adjustment is done). A consent notice will be registered on the title of a property alerting current and future property owners of certain obligations that must be complied with on a continuing basis by the owner, and subsequent owners, of a title. Consent notices can be varied or cancelled by agreement between the land owner and the Council at any time after the deposit of the survey plan.
Council	Whangarei District Council
Counterfort drain	A comparatively deep type of **subsoil drain** constructed by trenching and installing a perforated pipe at the base of the trench surrounded by a filter material to collect groundwater, and sealed at the surface to prevent capture of surface runoff. Counterfort drains are usually constructed parallel to the slope direction. The primary purpose is to reduce the groundwater level and decrease the pore water pressure, which increases the effective shear strength of the soil. Counterfort drains are typically between 2-5 m deep.
EES	Environmental Engineering Standards
Factor of Safety (FOS)	The FOS for geotechnical design of slope stabilisation measures is the ratio determined by dividing the resisting (stabilising) forces by the driving (destabilising) forces. A FOS < 1.0 indicates that a slope is unstable and will likely displace until equilibrium is reached (i.e. FOS = 1.0).
Hard engineering solution	Means of stabilising sloping ground or landslips by use of cut and fill earthworks, **slope buttresses** and in-ground structures such as **shear keys** or **palisade walls**.
Land stabilisation system	An engineered system designed to improve land stabilisation to provide an acceptable **FOS**. It may include **hard engineering** and/or **subsurface drainage** elements.
Palisade wall	A line of piles constructed below the ground level and extending into stable ground to improve stability often designed to intercept a **shear zone**.
Proponent	The owner of the subject land proposed to be developed.

TERM	DEFINITION
Shear key	Trench excavated into stable ground often below a **shear zone** and backfilled with material with high friction angle (generally granular material) to improve the stability of a slope or landslide.
Shear Zone / Shear Surface	A zone of ground (or surface) below ground level which is weaker than the surrounding ground and has either already developed a surface of movement or has the potential to develop a surface/zone of movement.
Slope buttress	A gravity structure or earth fill built at the toe of a slope to improve slope stability.
Subsoil drain	A drain comprising a perforated pipe constructed in a trench and backfilled with granular material, installed to collect subsurface or seepage water and convey it to a point of disposal. Subsoil drains have a variety of uses, including but not limited to; subgrade drainage below road pavements, drainage behind retaining walls, drainage at the base of gullies, drainage of saturated soils for agriculture and horticulture as well as drainage of slopes to improve slope stability.
Subsurface drainage	A drainage system constructed below ground to collect subsurface or seepage water and convey it to a point of disposal.
Underfill drains	Drainage measures typically constructed at the base of bulk earthworks fill and are typically located at depth below the final ground surface e.g. drains that follow cleaned out gullies or areas of seepage prior to starting bulk earthworks for subdivision developments.
Vested asset	An asset whose ownership is transferred to Council upon development completion.

3. Policy

Council's Policy is stated below:

1) Council will not accept private subsurface drainage systems for land stabilisation purposes as a vested asset.

2) Council will allow the use of private land stabilisation systems, subject to the following conditions:

   a) Engineering design and construction shall be in accordance with:

      i) Council's *Environmental Engineering Standards* (EES); and

      ii) Council's *Technical Standard: Stabilisation Systems for Land Development;*

      and shall be submitted for Council review and approval; and

   b) Subsurface drainage systems for land stabilisation purposes shall be placed clear of building platforms and structures, unless such drainage systems are part of the structure or serve a separate engineering function e.g. subsoil drains behind a retaining wall or road pavement subsoil drains.

   c) Underfill drains and hard engineering solutions may be located under building platforms and structures, as long as they do not interfere with or affect foundations or services; and

   d) Where subsurface drainage systems are placed in common trenches, other services shall be placed closest to the surface in order to be maintained without disturbing the subsurface drainage system. Where any such disturbance is required for maintenance then the trench shall be reinstated in a manner such that the subsurface drainage function of the trench is maintained; and

   e) The proponent shall provide construction records in accordance with Council's EES, clearly defining the location of all subsurface drains; and

   f) A consent notice shall be placed on each title benefiting from the private land stabilisation system, at the proponent's cost, with the following conditions:

i) The land owner or private entity (as defined in clause 3.2)f)ii)(1) below) shall be responsible for maintenance of the private land stabilisation system where required; and

ii) At Council's discretion:

(1) A private entity shall be established to monitor and manage the private land stabilisation system, including:

(a) A management plan including financing for maintenance and renewal of the private land stabilisation system; and

(b) An instrument requiring mandatory membership to the private entity outlined above.

(2) Conditions requiring ongoing reporting on the status and condition of the private land stabilisation system.

3) Maintenance structures and outlets for private land stabilisation systems that are proposed to be located in public reserves or Council property will require specific approval. For all such structures:

a) Council will evaluate the impact on the intended purpose of the reserve, and may refuse such occupation, or require additional consents and approvals; and

b) For structures approved by Council, the proponent at its own cost shall obtain from Council a licence to occupy in favour of the property, or private entity if one has been established in accordance with 3.2)f)ii)(1) above.

4. Deviations

Any deviations to this Policy will require the formal authorisation of the General Manager Infrastructure.

5. References

Tonkin & Taylor Ltd. (2018). Land Development Stabilisation – Technical Design Requirements

Whangarei District Council. (2010). Environmental Engineering Standards.

Whangarei District Council. (2011). Subsoil Drains Policy (superceded).

6. Adoption

This Policy has been approved for adoption by the Waste and Drainage Manager and the General Manager Infrastructure.

Andrew Carvell

Waste and Drainage Manager

Date

Simon Weston

General Manager Infrastructure

Date

Land Development Stabilisation – Policy FINAL.docx			
Audience (Primary)	External	**Business Owner (Dept)**	Waste & Drainage
Policy Author	Thane Richardt	**Next Review date**	December 2021

Policy adopted by Infrastructure Committee on DD-MM-YYYY.

Whangarei District Council

Land Development Stabilisation – Technical Design Requirements FINAL.docx

April 2018

Land Development Stabilisation – Technical Design Requirements FINAL.docx			
Audience (Primary)	External	**Business Owner (Dept)**	Waste & Drainage
Document Author	Tonkin & Taylor Ltd.	**Next Review date**	December 2021

1. Introduction

1.1 Purpose

The intent of this document is to specify design requirements for stabilisation systems for land development. It is to be read in conjunction with Council's Environmental Engineering Standards (EES) and the *Land Development Stabilisation - Policy*.

The technical content of this standard has been prepared by Tonkin & Taylor Ltd (T+T), in accordance with the Council's Land Development Stabilisation Policy document and with direction from the Council on acceptable levels of risk and future liabilities for Council related to land development. This document has been prepared by T+T for Council with reference to applicable professional standards, guidelines, procedures and practices at the date of issue of this document. Application and interpretation of this document in specific circumstances is outside the control of the Council and T+T and is the user's responsibility.

1.2 Scope

This document applies to private land development projects.

1.3 Glossary of Terms

The definitions provided in the table below relate to the general meaning and not to specifically required design details.

TERM	**DEFINITION**
AGS	Australian Geomechanics Society
AGS(2007c)	Refers to the "Practice Note guidelines for landslide risk management" published by the **AGS** in 2007 http://australiangeomechanics.org/admin/wp-content/uploads/2010/11/LRM2007-c.pdf
Building	As defined by the New Zealand Building Code.
Council	Whangarei District Council
Counterfort drain	A comparatively deep type of **subsoil drain** constructed by trenching and installing a perforated pipe at the base of the trench surrounded by a filter material to collect groundwater, and sealed at the surface to prevent capture of surface runoff. Counterfort drains are usually constructed parallel to the slope direction. The primary purpose is to reduce the groundwater level and decrease the pore water pressure, which increases the effective shear strength of the soil. Counterfort drains are typically between 2-5 m deep.
CPEng	Chartered Professional Engineer
EES	Environmental Engineering Standards
Extreme (worst credible) groundwater conditions	The groundwater levels assessed for a site under extreme conditions i.e. following significant rainfall events and/or due to failure of subsoil drainage elements and/or blockage of any downstream public stormwater system that the **subsoil drain** discharges into
Factor of Safety (FOS)	The FOS for geotechnical design of slope stabilisation measures is the ratio determined by dividing the resisting forces by the driving forces. A FOS < 1.0 indicates that a slope is unstable and will likely displace until equilibrium is reached (i.e. FOS = 1.0).

TERM	DEFINITION
Geo-professional	A Chartered Professional Engineer (**CPEng**) registered in the Geotechnical practice area with **IPENZ**, or a Professional Engineering Geologist (**PEngGeol**) registered with **IPENZ**.
GIS	Geographic Information System
Hard engineering solution	Means of stabilising sloping ground or landslips by use of cut and fill earthworks, **slope buttresses** and in-ground structures such as **shear keys** or **palisade walls**.
Horizontally bored drain	A drain drilled into a slope to reduce the groundwater level and decrease the pore water pressure. They usually consist of slotted PVC pipe installed at an upwardly sloping angle of inclination to the horizontal (normally 5°) to allow discharge by gravity. Horizontal drains can be installed in a fanned array arrangement or installed in rows parallel to the direction of slope.
IPENZ	Institution of Professional Engineers New Zealand
Land development	Land development refers to altering the landscape in any number of ways such as changing landforms from a natural or semi-natural state for a purpose such as agriculture or housing; subdividing real estate into lots; or changing the purpose of a land parcel.
Land stabilisation system	An engineered system designed to improve land stabilisation to provide an acceptable **FOS**. It may include **hard engineering** and/or **subsurface drainage** elements.
Normal long term groundwater conditions	The groundwater levels assessed for a site that would be expected under normal conditions including normal seasonal variations.
Palisade wall	A line of piles constructed below the ground level and extending into stable ground to improve stability, often designed to intercept a **shear zone**.
PEngGeol	Professional Engineering Geologist
Post-development	After bulk earthworks or final ground shaping (as applicable) has been completed for a particular site.
Proponent	The owner of the subject land proposed for development.
Public stormwater system	Stormwater pipe networks that are owned and maintained by Whangarei District Council.
Shear key	Trench excavated into stable ground often below a shear zone and backfilled with material with high friction angle (generally granular material) to improve the stability of a slope or landslide.
Shear Zone / Shear Surface	A zone of ground (or surface) below ground level which is weaker than the surrounding ground and has either already developed a surface of movement or has the potential to develop a surface/zone of movement.
Slope buttress	A gravity structure or earth fill built at the toe of a slope to improve slope stability.
Subsoil drain	A drain comprising a perforated pipe constructed in a trench and backfilled with granular material, installed to collect subsurface or seepage water and convey it to a point of disposal. Subsoil drains have a variety of uses, including but not limited to; subgrade drainage below road pavements, drainage behind retaining walls, drainage at the base of gullies, drainage of saturated soils for agriculture and horticulture as well as drainage of slopes to improve slope stability.
Subsurface drainage	A drainage system constructed below ground to collect subsurface or seepage water and convey it to a point of disposal.

TERM	DEFINITION
Underfill drains	Drainage measures typically constructed at the base of bulk earthworks fill and are typically located at depth below the final ground surface e.g. drains that follow cleaned out gullies or areas of seepage prior to starting bulk earthworks for subdivision developments.
Vested asset	An asset whose ownership is transferred to Council upon development completion.

1.4 Background

Subsurface drainage is a common method used by Geo-professionals to improve the FOS of a slope. NZS 4404:2010 and NZS 4431:1989 include some subsurface drain design requirements[1] for subdivisions and residential developments which are, in general, applicable to the design of subsurface drains for slope stabilisation. The relevant sections are summarised below.

Section 4.3.9.9 "Subsoil drains" of NZS 4404:2010 states:

> *"Subsoil drains are installed to control groundwater levels. Perforated or slotted pipe used under all areas subject to vehicular traffic loads shall comply with NZTA specification F/2 and NZTA F/2 notes. It is good practice to provide regular inspection points.*
>
> *Bedding and backfill material around a subsoil drain pipe shall be more free-draining than the in situ soil. If filter fabrics are used their susceptibility to clogging, thereby reducing the through flow, should be considered."*

Section 6.2.4 "Subsoil drainage" of NZS 4431:1989 states:

> *"6.2.4.1*
>
> *Before fill is constructed over natural ground, pervious drains or similar subsoil seepage control systems should be installed to lead seepage away from all springs or potential areas of seepage emission from natural ground into fill, in order to:*
>
> *(a) Prevent saturation of the fill before construction of the fill is complete (prior saturation can delay settlement of the fill);*
>
> *(b) Prevent internal erosion ("piping"); and*
>
> *(c) Prevent internal seepage pressures which reduce shear strength.*
>
> *6.2.4.2*
>
> *Subsoil drains shall discharge via flexible jointed pipes to a destination approved by the local authority, preferably to stable watercourses or to piped stormwater systems.*
>
> *6.2.4.3*
>
> *A record shall be kept of the position, type and sizes of all subsoil drains, and in particular, the position of their outlets.*
>
> *6.2.4.4*
>
> *Where seepage is encountered from a sloping natural ground abutting a fill, a bench shall be cut just below the line of seepage and drains installed to collect the seepage and discharge it clear of the fill to a destination approved by the local authority, preferably to a stable watercourse or a piped stormwater system."*

There is risk associated with relying solely on subsurface drainage as the only means of slope stabilisation. Subsoil drains can over time become less effective, or in extreme cases become blocked. This depends on a number of factors, some of which may be able to be controlled during the design stage (e.g. use of filters, good specification, type of drain suited to the ground conditions/permeability) and other factors where there is less

[1] NZS 4404 and NZS 4431 use the phrase "subsoil drain". For the purposes of section 1.4 the terms "subsoil drain" and "subsurface drain" are used relatively interchangeably; elsewhere in this technical standard a *subsoil* drain is defined as a type or subset of *subsurface* drain.

control post-construction (e.g. algae growth, reliance on maintenance, protection from future development etc.). Some of the design and construction considerations are referred to in the NZTA F2 Specification. However, neither NZS 4404:2010, NZS 4431:1989 or NZTA F2 cover all the issues (particularly post-construction) for design of subsurface drainage for slope stabilisation purposes.

The Geo-professional should consider the limitations of particular systems when undertaking stabilisation design. For example, horizontally bored drains are limited in their capacity to reduce groundwater levels, due to their reduced cross sectional area. They are also at greater risk of blockage compared to counterfort drains and would not normally be installed as a standalone measure to stabilise land.

2. Design Requirements

2.1 General

The design of land stabilisation systems shall be performed and certified by a competent Geo-professional, and shall include a site-specific geotechnical risk assessment for both the existing and developed conditions.

In addition to the requirements of this document any stabilisation systems for land development shall also comply with the EES.

Land stabilisation systems shall be designed for at least 100 years' life.

2.2 Geotechnical Risk Assessment

Broad-scale geotechnical hazard maps are published on Council's online Geographic Information System (GIS) and identify areas prone to landslide hazards for planning purposes[2]. When land is proposed to be developed, the Geo-professional shall undertake a site-specific qualitative geotechnical risk assessment to characterise the slope stability hazard and the consequence of slope failure, and assign a risk classification in accordance with Appendix C of (AGS(2007c), 2007). This will refine the broad-scale three-category hazard classification into a site-specific five-category risk classification.

2.3 Design solution

Following the geotechnical risk classification described in section 2.2 above, the Geo-professional shall prepare a design solution incorporating the requirements of Table 2-1,

Figure *2-1* and Table 2-2.

The risk assessment shall continue through the slope stabilisation design process and consider the consequence of partial or complete failure of subsurface drainage and the effect on the proposed development. The outcomes of the risk assessment shall be included in the geotechnical design report and presented to Council in support of any request for engineering approval.

For sites assessed to have a High to Very High Risk level, subsurface drains will not be accepted as a standalone measure for slope stabilisation due to their potential to reduce in effectiveness over time. Acceptable design solutions are further discussed below (refer to

Figure *2-1* and Table 2-2). The minimum FOS to use in the design of land stabilisation systems is stated in Table 2-1.

Table 2-1: Minimum FOS for land stabilisation design

Design Condition	Minimum FOS
Normal long term groundwater conditions	1.5

[2] Hazard maps and accompanying reports are available on Council's website www.wdc.govt.nz

Extreme groundwater conditions	1.3
Seismic condition in 500 year return period event	1.1

2.3.1 Case 1

When the post-development FOS is less than 1.3 for normal groundwater conditions, hard engineering solutions (with or without subsurface drainage) are required to improve the FOS to ≥ 1.5. If subsurface drainage is used, the design will also need to confirm that a sufficient margin of safety is retained under extreme groundwater conditions, assuming drainage failure.

2.3.2 Case 2

When the post-development FOS is greater than 1.3 for normal groundwater conditions, subsurface drainage may be used to improve the FOS to ≥ 1.5. The design will also need to confirm that a sufficient margin of safety is retained under extreme groundwater conditions, assuming drainage failure.

Land Development Stabilisation – Technical Design Requirements FINAL.docx			
Audience (Primary)	External	**Business Owner (Dept)**	Waste & Drainage
Document Author	Tonkin & Taylor Ltd.	**Next Review date**	December 2021

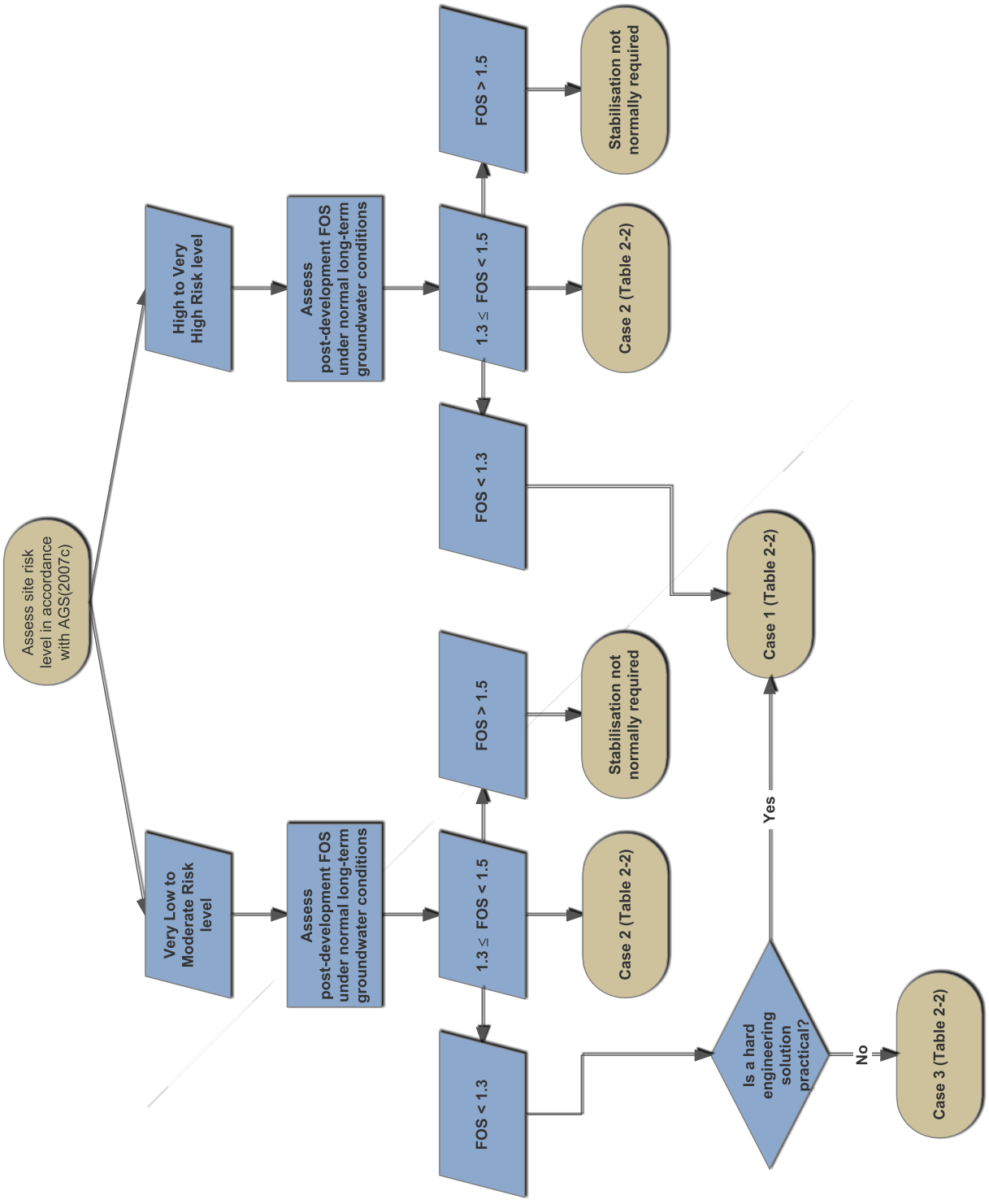
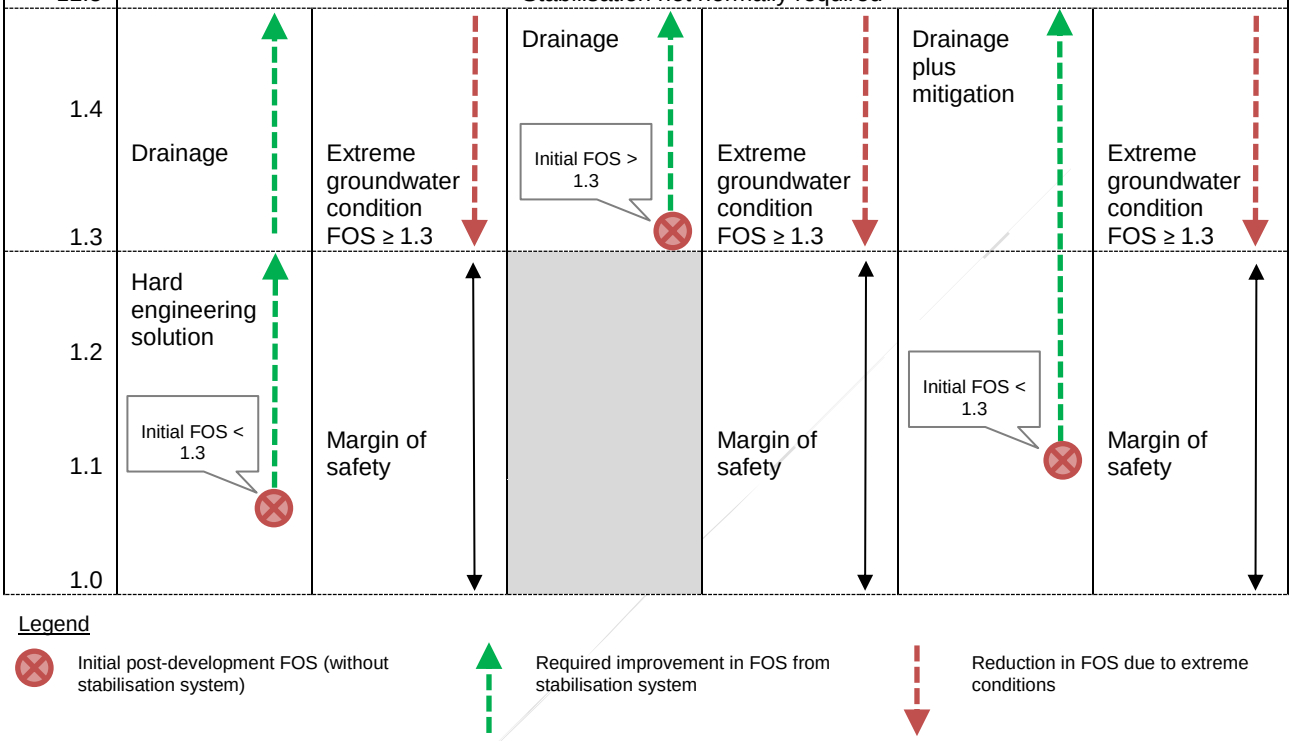


Figure 2-1: Process for selecting a land stabilisation system

Table 2-2: Stabilisation solutions

Factor of Safety	**Case 1** Hard engineering solution with or without reliance on subsurface drainage		**Case 2** Subsurface drainage only(3)		**Case 3** Subsurface drainage plus mitigation measures(3)	
	Post-development FOS (normal conditions)	Post-development FOS (extreme conditions)	Post-development FOS (normal conditions)	Post-development FOS (extreme conditions)	Post-development FOS (normal conditions)	Post-development FOS (extreme conditions)
≥1.5	Stabilisation not normally required					
1.4 – 1.3	Drainage	Extreme groundwater condition FOS ≥ 1.3	Drainage	Extreme groundwater condition FOS ≥ 1.3	Drainage plus mitigation	Extreme groundwater condition FOS ≥ 1.3
1.2 – 1.1	Hard engineering solution	Margin of safety		Margin of safety		Margin of safety
1.0						



Notes

1. Figure 2-1 for applicability of various cases.
2. Refer to Table 2-1 for minimum FOS.
3. Doesn't preclude the use of hard engineering solutions.

2.3.3 Case 3 (Very Low to Moderate Risk only)

This case will only be considered for sites assessed as very low to moderate risk in accordance with section 2.2.

If it can be demonstrated to Council's satisfaction that a hard engineering solution is impractical, Council may at its sole discretion consider a subsurface drainage solution only, provided a whole-of-life strategy is developed including the following mitigation measures:

a) All constraints to the use of hard engineering solutions should be re-examined by the proponent and Council before implementing a subsurface drainage-only solution. Where consent conditions are constraining or precluding the use of a hard engineering solution then consideration should be given to relaxing those conditions;

b) The Geo-professional shall demonstrate that an appropriate margin of safety is retained if the subsurface drains fail under extreme groundwater conditions;

c) Adequate redundancy is built into the drainage design i.e. more drainage than required, different types of drainage, multiple outlets etc.;

d) The design allows for regular cleaning/flushing and that regular maintenance is enforced and carried out e.g. via a covenant on the title (and the liability for failing to maintain is explicitly defined);

e) The design provides suitable access for maintenance, as described in d) above;

f) Specification for the drainage is set high to reduce the risk of blockage e.g. conservatively sized pipe, concrete outlets, high specification drainage metal etc.;

g) Design certification by a suitably qualified and experienced Geo-professional in accordance with the EES;

h) Contingency measures are identified in the case that the drains become blocked e.g. designing so they can be replaced or flushed if required;

i) Ongoing monitoring of groundwater levels and pore water pressures by means of in-ground piezometers. Monitoring is enforced similar to item d) above; and

j) A Management Plan is prepared to capture items d) and i).

2.4 Location of Land Stabilisation Systems

Land stabilisation systems shall be located in accordance with the *Land Development Stabilisation - Policy.*

3. Design Submissions

3.1 General

The required content of geotechnical assessments is defined in the EES.

3.2 As-Constructed Drawings

As-built drawings shall record the position, depth, type and dimensions of all land stabilisation system components, and in particular the position and depth of subsurface drain pipes and outlets. Submissions shall additionally comply with EES requirements.

4. References

AGS(2007c) Practice Note guidelines for landslide risk management [Journal]. - [s.l.] : Australian Geomechnics, 2007. - 1 : Vol. 42.

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Tonkin & Taylor Ltd Coastal Structure Plan - Slope instability hazard potential and effluent disposal potential Oakura to Langs Beach [Report]. - 2005.

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Whangarei District Council Stabilisation Systems for Land Development Policy (Superceded). - 2016.

Land Development Stabilisation – Technical Design Requirements FINAL.docx			
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Document Author	Tonkin & Taylor Ltd.	**Next Review date**	December 2021

5. Adoption

This Technical Standard has been authorised by the Waste and Drainage Manager and the General Manager Infrastructure.

Andrew Carvell

Waste and Drainage Manager

Date

Simon Weston

General Manager Infrastructure

Date

Australian Geomechanics Society

Extract from

Australian Geomechanics

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"Practice Note Guidelines for Landslide Risk Management 2007"
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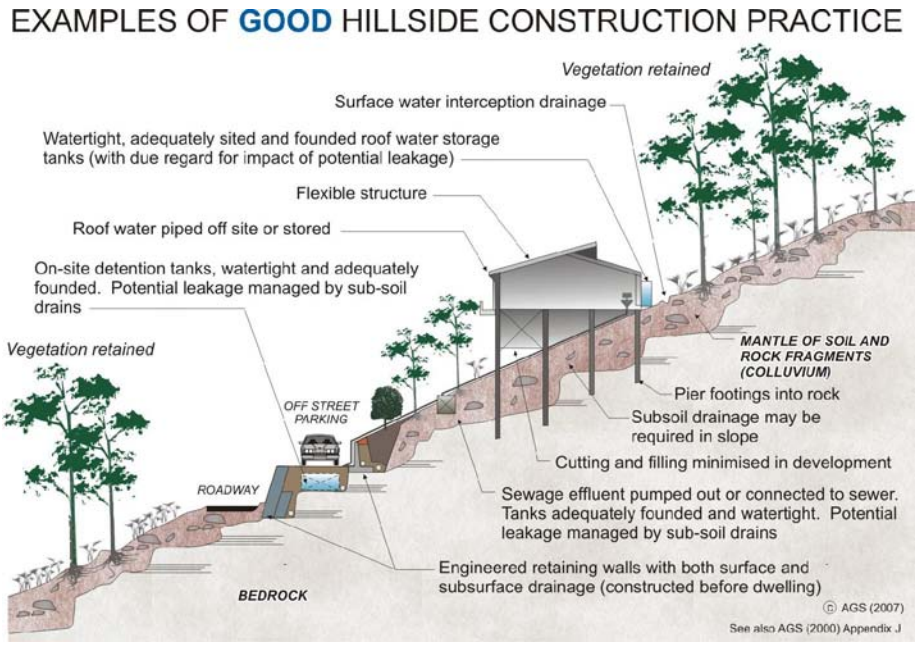

Landslide Risk Management

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PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

Australian Geomechanics Society Landslide Taskforce,

Landslide Practice Note Working Group

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PART A: BACKGROUND

1 INTRODUCTION

1.1 PREAMBLE

Slope instability occurs in many parts of urban and rural Australia and often impacts on housing, roads, railways and other development. This has been recognised by many local government authorities, and others, and has led to the requirement by many local government councils for stability assessments prior to allowing building development.

In 2000, the Australian Geomechanics Society (AGS) published "Landslide Risk Management Concepts and Guidelines" (AGS 2000). Since then there have been many published papers and discussion which have progressed Landslide Risk Management (LRM) in particular and risk management in general. As a consequence, AGS considered it appropriate to develop more comprehensive guidelines for practitioners and regulators involved in LRM.

This Practice Note Guidelines for Landslide Risk Management (the Practice Note) and its Commentary (AGS 2007d) are one part of a series of three guidelines related to LRM that have been prepared by AGS with funding under the National Disaster Mitigation Programme (NDMP). That programme has been introduced by the Australian Government to fund disaster mitigation, addressing hazards such as flooding, bushfires and landslides.

The associated guidelines which should be read in conjunction with the Practice Note are:-

- AGS (2007a) *"Guideline for Landslide Susceptibility, Hazard and Risk Zoning for Land Use Planning"*.
- AGS (2007e) *"Australian GeoGuides for Slope Management and Maintenance"*.

1.2 PURPOSE

The purpose of this Practice Note is to:

1. Review the Australian Geomechanics Society (AGS) Landslide Risk Management Concepts and Guidelines (AGS 2000) in the light of usage since publication and update accordingly and in addition, to take the opportunity to establish a formal revision process/documentation. Accordingly, a Revision Table is included in the Practice Note.
2. Provide guidance and recommendations on tolerable risk criteria, minimum reporting standards and assessment criteria/options to Local Government and Government bodies who as the regulator, receive Landslide Risk Management (LRM) reports and decide on levels of Tolerable Risk.
3. Provide guidance of a technical nature in relation to the processes and tasks undertaken by geotechnical practitioners who prepare LRM reports including appropriate methods and techniques. The Practice Note is a statement of what constitutes good practice by a competent practitioner for LRM, including defensible and up to date methodologies.
4. Provide guidance on the quality of assessment and reporting, including the outcomes to be achieved and how they are to be achieved. It sets out the functions and responsibilities of the professional carrying out the assessment.
5. Be a reference document for legislative purposes, which has been subject to nation-wide peer review.

1.3 SCOPE

This Practice Note supersedes AGS (2000) as the guideline for good practice and is accompanied by a Commentary (AGS 2007d) which discusses various aspects and gives appropriate references, and which should be read in conjunction with this Practice Note.

AGS (2000) contains much useful and relevant commentary which can (and should) be read in conjunction with the Practice Note. It is not the intention of the Practice Note to supersede this valuable commentary, rather to complement it. AGS (2000) should be regarded as "companion literature". Unless specifically discussed or revised in the Practice Note, the Working Group considers the commentary, examples and references provided in AGS (2000) to constitute appropriate background for the use of the Practice Note.

The emphasis of the Practice Note is on residential subdivision and development, particularly when considering the requirements for assessment on a lot-by-lot basis for either existing or proposed development.

The recommendations are however applicable to all classes of urban and rural building development or the environment.

The risk analysis principles could be adopted for short term risks associated with trenches or excavations during construction projects and for quarries and open cut mines. For such cases, risk tolerance criteria are controlled by occupational health and safety requirements and are not covered here.

The Practice Note can be applied to roads and railways. However, special consideration has to be given to the number of users, their temporal spatial probability and the summation of the risk along the route. This is discussed further in the Commentary.

1.4 CONVENTIONS USED

The Practice Note includes imperative verbs, such as 'establish', 'use', 'identify' and so on. These are to be understood as meaning; *"AGS recommends that you establish…"*, or *"…that you use…."* or *"…that you identify….."* and so on as the case may be. This form of expression has been used to avoid unnecessary repetition of wording in the sense of 'plain English'.

Paragraphs presented in **bold type** constitute the guideline statement and subsequent sub paragraphs provide discussion of the guideline topic. Further discussion is provided in the Commentary.

In the following, use of the word 'landslide' implies both existing (or known landslides) and potential landslides which a practitioner might reasonably predict based on the relevant geology, geometry and slope forming processes. Such potential landslides may be of varying likelihood of occurrence. 'Landslide' also includes 'landslip' (as used in Victorian legislation), 'slump' and the various landslide forms (see Appendix B).

1.5 STAKEHOLDERS

The various stakeholders who may be affected by landslide risk include:-

- The **landowner** who will frequently be the client in terms of a commission to prepare a LRM report for a site or a development proposal.
- The **occupier** who would most often also be the land owner.
- The **financier** who would often be a financial institution having an interest in the land and any development thereon.
- The **regulator** (Appendix A) who would have responsibility for setting risk acceptance criteria, administering planning controls and approving development proposals as being within the requirements of planning controls, or a policy.
- The **practitioner** (Appendix A) who would have the required expertise for and responsibility of preparing a LRM report and recommending suitable risk control measures, when needed, to achieve the risk acceptance criteria.
- The **design professional** (such as architect or structural engineer) who would be one of the advisors to the client with responsibility for integration of risk control measures recommended by the practitioner into the development scheme, where possible, within the design brief from the client.
- The **insurer** where appropriate may have an interest in providing insurance cover against nominated insurable risks.

Although there is no section in the Practice Note dealing with the Client, clearly the Client is an essential stakeholder in relation to the practitioner. The Client will be relying on unbiased, sound technical advice from the practitioner as to the risk that a development proposal poses to the client and /or his interests. It will be the responsibility of the client to accept the risks involved, subject to the approvals of the regulator.

2 RISK TERMINOLOGY

The framework for the LRM process, as shown in Figure 1 in a simplified flow chart form, should be adopted.

Adopt the recommended terminology for ease of communication and clarity as defined in Appendix A.

As with most areas of expertise, there is a technical jargon associated with LRM. Specialist terminology is used to convey succinct ideas or facts. This cannot be avoided and by necessity is of a technical nature. The relevant terminology is defined in Appendix A. The lay reader is also referred to the Commentary for further discussion and to the GeoGuides (AGS 2007e).

This Practice Note, and the companion AGS guidelines (AGS 2007a, 2007e), use the term 'landslide' rather than 'landslip' or 'slump' or similar, to cover a wide range of failure mechanisms in soil, rock (as discussed in Appendix B) and man made structures such as retaining walls, as implied by the definition in Appendix A.

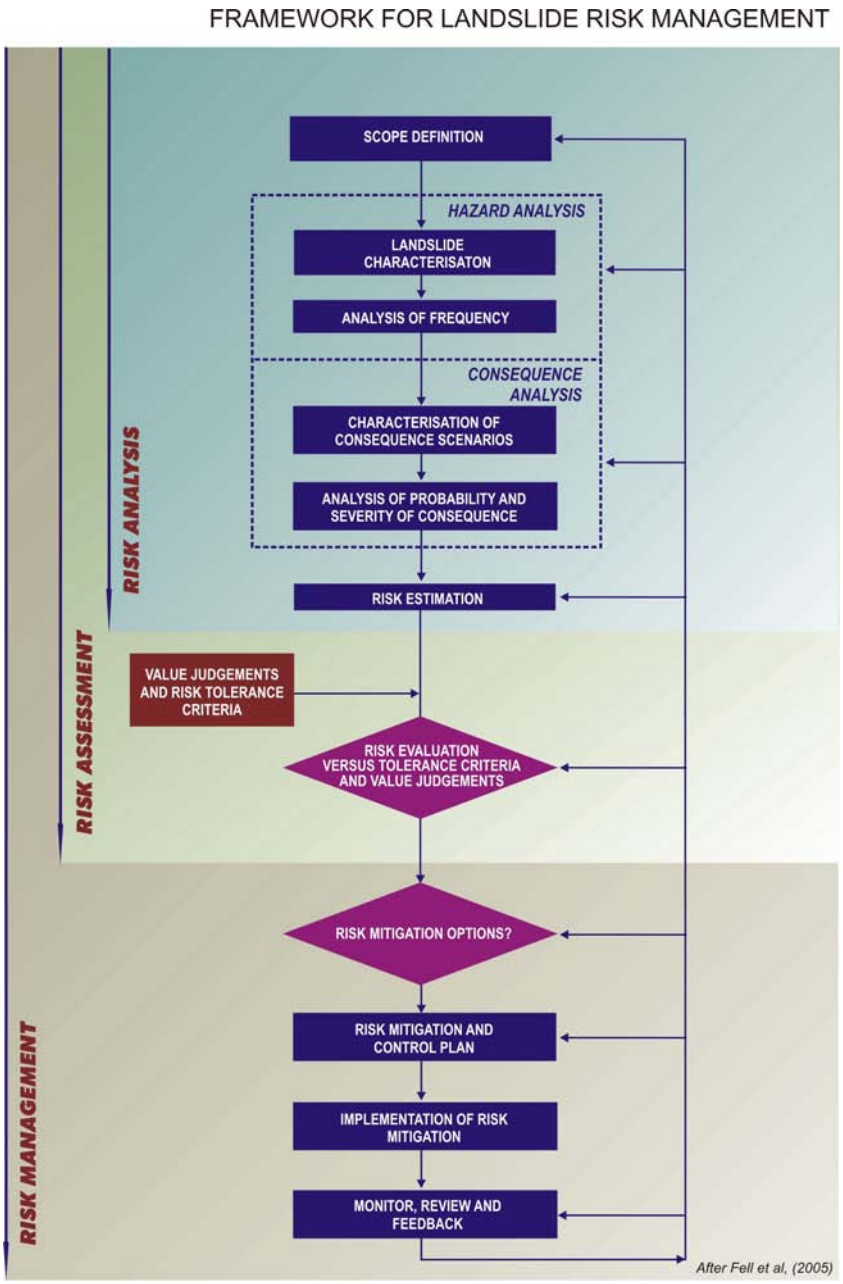


Figure 1.

The Framework for LRM presented in Figure 1 is similar to the flow chart in AGS (2000). However, it has been simplified in presentation and has been amended slightly from AGS (2000) to reflect the inclusion of Frequency Analysis as part of Hazard Analysis (in accordance with the abovementioned definition of hazard and as defined in AGS 2000).

Definitions for associated terminology have also been included in Appendix A together with an explanation of Landslide Risk as presented in AGS Australian GeoGuide LR7.

PART B GUIDELINES FOR REGULATORS

3 GUIDELINES FOR REGULATORS

3.1 BACKGROUND

The term landslide denotes *"the movement of a mass of rock, debris or earth down a slope"*. The phenomena described as landslides are not limited to either "land" or to "sliding" and usage of the word has implied a much more extensive meaning than its component parts suggest. The rates of movement cover the full range from very rapid to extremely

slow. The size, similarly, can vary enormously. The combination of type of landslide, size and rate of movement can determine the destructive power, and hence potential consequences of the landslide in terms of damage to property, loss of life, economic costs and impact on the environment. Subsidence, as a mechanism, is excluded from consideration, though it may be similar in consequence and appear to be of a similar form. Appendix B presents a summary of the terminology used to classify and describe landslides.

Landslides can impact on human development and activity as well as natural areas / features. It is the potential impact on human development which becomes of concern to the planners, regulators and disaster management authorities. Landslides can be just one of a number of threats which have to be considered, others being for example flooding, bush fires, and seismicity.

Examples of where landsliding is potentially an issue include:-

a) Where there is a history of landsliding.
b) Where there is no history of sliding but the topography dictates sliding may occur.
c) When there is no history of landslides but geological and geo-morphological conditions are such that sliding is possible.
d) Where there are constructed features which, if they fail, may travel rapidly.
e) Forestry works and agricultural land clearing which can lead to landslides causing damage to the environment.

Specific examples of the above are given in the AGS Guidelines for Landslide Susceptibility, Hazard and Risk Zoning for Land Use Planning (AGS 2007a). AGS (2007a) also provides detailed guidance to the regulator in relation to landslide zoning for planning purposes.

3.2 RELEVANCE TO APPROVALS PROCESS

Details of the approvals process may vary in detail from state to state. It is understood that in all States and Territories of Australia, the regulator has a statutory responsibility to consider the impact of a number of hazards, including landslides, on potential development of land as a 'duty of care' exercise. The regulator is usually the local government, but may be a State Government department or body. The actual mechanism and regulatory context for dealing with planning controls, building controls and approval process varies from state to state. However, the outcome should be that areas having a landslide risk are properly considered in relation to land use and development proposals.

In order to develop planning controls and building regulations, local government (or other regulators) must ensure that it has the statutory means to:

a) Through a planning scheme and using the principles in AGS (2007a), identify the areas that are susceptible to or at risk from landslides.
b) Require planning and/or building approvals for all land use and development within the areas zoned as susceptible to landslides.
c) Ensure there is a proper process for assessment in relation to existing and proposed development, including the requirement for completion of LRM reports in accordance with this Practice Note.
d) Provide appropriate risk tolerance criteria for loss of life and property so that there is a means to determine whether it is appropriate for development to occur or the required land use to proceed.
e) Apply, if necessary, consent conditions on the land use and/or development approval, including conditions requiring maintenance that will appropriately manage the landslide risk for that use and/or development.

It can be seen from the above that zoning in accordance with AGS (2007a) becomes the 'initiator' under the planning scheme and building approvals process to determine whether LRM controls are required and whether more detailed LRM consideration is required.

3.3 POLICY REQUIREMENTS

The regulator should have a specific policy which sets out the requirements for LRM assessments as part of the development application documentation and process.

The need for such a policy should be determined by zoning studies in accordance with AGS (2007a). Essential components of such a policy will include:

3.3.1 **When a LRM assessment is required.** This may be related to a Susceptibility or Hazard Zoning Study or some other plan or criteria defining areas or types of development included or excluded.

3.3.2 **The necessary competencies of practitioners undertaking LRM assessments**. **Such practitioners should be required to have LRM as a core competency.** A method of demonstrating core competency in LRM is being addressed by the Australian Geomechanics Society and Engineers Australia as a specific area of practice within the National Professional Engineers Register (NPER). Some regulators may choose to define another method of demonstrating competency.

3.3.3 **The basic requirements of LRM reports** which should be based on compliance with the requirements of this Practice Note.

3.3.4 Require assessment of risk to life as part of a LRM report which, as discussed below, should be completed in a quantitative basis.

3.3.5 Suggest adoption of the preferred qualitative terminology given in Appendix C of this Practice Note for risk to property so that the regulator can become accustomed to the terminology adopted and implications arising there from. If alternative terminology is to be adopted for LRM, the regulator should only accept non standard schemes where the terms have been clearly defined, the terms have been explained in relation to the preferred terminology and it can be reasonably demonstrated by the practitioner that the alternative is better suited to the particular circumstances of the assessment.

3.3.6 Provide the required forms to control the submissions and approvals process.

3.3.7 Specify the criteria under which a decision will be made for both the scope/nature of developments and the appropriate tolerable risk criteria being adopted.

3.4 PROCESSING REQUIREMENTS

3.4.1 The regulator should use a number of forms to provide appropriate QA process control and documentation records of the submitted LRM assessment and subsequent compliance with the approval conditions.

The forms need to be appropriate to each stage of the development application, approval, detailed design, construction and maintenance of the development. Essential contents will include:

1. Name and qualification of the practitioner responsible for the LRM assessment.
2. A list of supporting documents including the architectural, civil design and structural engineering design drawings, as appropriate, to fully define the extent and scope of the proposed development.
3. A statement of compliance with the requirements of this Practice Note. In some cases the statements will be required to include details of how compliance is achieved.
4. Document reference details (date, reference number, report title) for the relevant LRM assessment submission.

A suite of example forms is given in Appendix D for modification by each regulator to be consistent with their policy. The aim of the forms is to provide appropriate documentary control of the stages required through to completion of a development.

Processing of the application by the regulator should include, amongst other aspects, confirmation that the submission is in accordance with policy requirements, and that the nature of the development complies with the requirements of the LRM assessment.

Where the regulator has specific concerns in relation to the adequacy of a submission, or the conclusions reached, or if required by a Hazard Zoning study, the submission may be subject to peer review or independent specialist advice to the regulator as an audit process or as part of mediation for an agreement. The reviewer should independently review the LRM assessment report in terms of adequacy of compliance with this Practice Note and the reasonableness of the assessment conclusions and risk control measures specified. The review should also consider the specific development proposals as defined by the design drawings.

3.4.2 Where the recommendations of this Practice Note have not been followed, then the regulator should <u>either</u> reject the application <u>or</u> require provision of further information before approval is given.

It is anticipated that the forms in Appendix D will, in part, constitute a checking template for the regulator. Further discussion is given in the Commentary.

3.4.3 Where construction is completed but all aspects of the Approval Conditions have not been completed with appropriate documentation or justification, then the final approval by the regulator should not be given until sufficient information is provided to demonstrate compliance.

It is anticipated that completion of Forms F and G with suitable annotation would help identify where non compliance exists. If the regulator does not have a strong procedure for enforcement of, or auditing of, compliance with consent conditions, then there may be subsequent liability issues for the regulator if non-compliance becomes an issue at a later date.

3.5 ESTABLISHMENT OF TOLERABLE RISK CRITERIA

The regulator is responsible for setting the Tolerable Risk Criteria for loss of life and property loss. Discussion of the considerations and world practice are given in the Commentary together with the AGS recommendation for consideration by the regulator.

3.6 LANDSLIDE INVENTORY

The local Council, or other regulator, should maintain an inventory of past landslide events as discussed in AGS (2007a) and make this information available to all practitioners.

3.7 ROLE AND RESPONSIBILITY OF THE PRACTITIONER

The practitioner has the role of providing technical input in relation to the specialized aspect of LRM. Such input will be subject to the specific requirements of any policy instituted by the regulator. The regulator may require specific levels of qualification and competence of practitioners providing the regulator with advice in relation to compliance with the risk acceptance criteria.

The qualifications and experience of suitable practitioners are as discussed in Paragraph 3.3.2.

It is the responsibility of the practitioner to carry out LRM assessments in accordance with this Practice Note and within the requirements of his/her professional Code of Ethics. The practitioner must provide advice to the client and regulator in an unbiased manner.

PART C GUIDELINES FOR PRACTITIONERS

4 SCOPE DEFINITION

Establish the purpose and scope of the risk assessment study.

The practitioner needs to take into account the initial brief from the client and the requirements of the regulator. Usually these will be sufficient for the practitioner to decide on the appropriate scope and level of the study which should then be advised to the client as a "reverse brief". In the LRM process, the practitioner will have a role to advise the client as to how the landslide risk can be reduced, avoided or otherwise controlled including options or alternatives.

5 HAZARD ANALYSIS

5.1 DATA GATHERING / DESK STUDY

Assemble relevant data and record their sources.

Often there is a body of local experience which becomes invaluable for the assessment process. Such experience includes published papers, geological maps, aerial photographs and general studies such as Hazard Zoning studies completed for the regulator. Local experience can include previous assessments and knowledge of problematic areas which should be available from the regulator's landslide inventory. Practitioners new to an area should discuss with locals their knowledge and experience.

Preferred data for the assessment will include site specific data, such as survey plan showing existing features, spot heights, contours and location and nature of services. Initial design proposals are required so that the risk assessment may be completed and appropriate risk control measures specified. (It is a necessary requirement in the performance of a risk assessment for there to be an element at risk, hence the need for a preliminary design or for an assumed development which should be defined in the LRM report).

5.2 FIELD INVESTIGATION REQUIREMENTS

5.2.1 Complete investigations sufficient to establish a geotechnical model, identify geomorphic processes and associated process rates.

The investigation may involve a number of methods and may be completed in stages, with each stage sufficiently detailed to provide a model appropriate to the level of study being undertaken. Further discussion is given in the Commentary.

5.2.2 Inspect the site and surrounds including field mapping of the geomorphic features.

This must be completed by the practitioner for every assessment. The field mapping is to document the observations and to enable formulation of the geotechnical model.

Mapping should be completed to scale on an available survey plan and must include the surrounds (above, below and adjacent) to the site as appropriate to define the landslides and the geotechnical model.

Where a survey plan is not available, then simple survey using hand held tape and clinometer methods should be used to draw up a plan, to scale, using standard mapping symbols and terminology to represent the geological and geomorphic features. (Examples of geological and geomorphic mapping symbols are presented in Appendix E.)

5.2.3 Determine the subsurface profile from exposures or subsurface investigation such as by boreholes and/or test pits.

This is necessary as part of the geotechnical model. Often exposures or knowledge from a nearby site may be sufficient.

Where such data is not available or not appropriate, subsurface investigation is required to enable formulation of the model and must include determination of the depth to rock or to below the depth of potential failure surfaces if this is greater.

Where pre-existing landslides are expected or suspected, then where practical, use should be made of either test pits (to enable sufficient sample/material to be seen for identification of shear planes or other relevant structure) or boreholes (with appropriate sampling and installation of inclinometers for monitoring for evidence of movements).

5.2.4 Assess likely groundwater levels and responses to trigger rainfall events.

Consideration of the likely ground water response will enable assessment of response to rainfall trigger events. Use may be made of experience in the area, as observation of site specific data will frequently require prolonged periods of monitoring to enable formulation of a groundwater response model taking into account the statistical significance of rainfall events during the monitoring period. For relatively straightforward projects with low to moderate risks, a basic qualitative estimate of groundwater levels and responses may be appropriate when there is a lack of data. However, other more complicated projects, or where risk levels are higher, will require a greater level of understanding of groundwater levels and responses.

For more detailed analysis, particularly of possible stabilisation measures by subsurface drainage, observation of groundwater levels and their response to significant rainfall events is advisable to enable subsequent assessment of the effectiveness of subsurface drainage measures. Careful consideration must be given to the location of piezometers and their construction details.

5.2.5 Prepare a cross section drawing (to scale) through selected parts of the site to demonstrate the geotechnical model of site conditions and on which landslides may be identified.

The resulting geotechnical model should integrate all the data obtained from the mapping and investigations.

The section should demonstrate the likely variation in subsurface conditions on the section including groundwater levels. On large or complex sites, more than one section may be required. All sections are to be drawn to natural scale. If exaggerated vertical scale is required for clarity, then a summary section at natural scale should also be included.

Adequate investigation has been completed when the geotechnical model is sufficiently defined to understand the slope forming processes relevant to the site and surrounds, the form and extent of landslides, likely triggers for the landslides and process rates associated with the landslides. The report should include explanation of uncertainties associated with the model.

5.2.6 Take into account slope forming process rates associated with the geotechnical model and landslides.

An understanding of the slope forming process relevant to the landslides and associated process rate is fundamental for evaluation of likelihood.

5.2.7 Identify landslides types/locations appropriate to the geotechnical model based on local experience and general experience in similar circumstances.

The types of landslides will be dependent on the geotechnical model and to some extent on the nature of existing and/or proposed development. The expected characteristics of the landslides (such as the size, type of material involved, rate of failure and travel distance) need to be assessed. The range of landslide sizes can vary from the very large landslides, which may encompass a whole hillside or region, to a small site specific landslide. The model should include assessment of the fundamental cause as well as likely trigger events. The report must document the hazard assessment which will include the estimated likelihood for each landslide type.

The hazard assessment must address areas upslope from the site, downslope from the site and across the slope adjacent to the site where these may affect the site.

5.2.8 If required, further detailed investigations should be completed to better define the model, the landslides, the triggers, the frequency (likelihood) or design of stabilisation measures to control the risk.

Such additional investigation is most likely to be required on sites where the risk is judged to be intolerable and/or where further input is required to resolve uncertainties**.**

5.3 LANDSLIDE CHARACTERISATION

Characterise the landslides based on the desk study and field investigations. Use Appendix B for terminology to describe the landslides.

The characterization should include the classification, volume, location and potential travel distance of all landslides which may occur on the site or travel on to or regress into the site.

5.4 FREQUENCY ANALYSIS

5.4.1 Techniques for Frequency Analysis

a) Adopt a frequency analysis technique appropriate to the level of study and complexity of the geotechnical model and slope forming process.

The appropriate technique may change with different levels of study, or for different stages of a project, or with the project brief and available budget. For example, techniques and level of detail may be different for:

- Subdivision stage LRM
- Residential dwellings LRM
- Infrastructure and utilities LRM
- Natural resource and environmental LRM

It is essential that the assessment be based on the best estimates available and that expert judgment be applied to answers so derived.

It is essential to understand the slope forming process before moving on to the frequency assessment.

The assessment must document the reasoning in a transparent manner.

b) Gather local and historical knowledge of slope performance and landslide characteristics and occurrence. The resulting inventory enables assessment of frequency.

This technique is a basic starting point and essential for all studies. However, a common shortcoming is that "local knowledge" is often poorly documented and difficult to collate and assess. Local Council records and experience should be accessed via a landslide inventory made available to practitioners. Analysis of aerial photographs and possibly maps may provide additional data.

Documentation of events by local newspapers may also be a useful source, depending on the quality of reporting and what events are judged at the time to be of local interest.

c) Empirical methods based on slope instability ranking systems.

These methods are often devised by expert groups to assist with prioritisation of treatment measures.

The methods are usually based on subjective judgment of the relative importance of contributory factors. The results obtained may be difficult to calibrate or it may be difficult to obtain consistent results and hence may be inaccurate. The methods do not usually allow assessment of frequencies.

d) Relationship to geomorphology and geology.

This method is based on the principle put forward by Varnes (1984) that the past and present are guides to the future. Hence, this leads to the assumptions that:

1. it is likely that landsliding will occur where it has occurred in the past and
2. landslides are likely to occur in similar geological, geomorphologic and hydrological conditions as they have in the past.

The use of historic records and landslide inventories of past performance are likely to be required to enable frequency values to be assessed. However, it should be noted that landslide frequency, size and intensity may differ from past performance where altered trigger events are introduced, e.g. due to man made changes or climate change. In addition, other factors (such as periodic or seasonal wetting and drying cycles resulting in soil creep, cyclic degradation and strength loss) can also result in failures after relatively "normal" rainfall events.

The use of other slope attribute factors (such as slope angle, slope drainage, slope age, presence of groundwater, slope orientation) may assist with assessment of particular slopes relative to the broad geomorphic model.

e) Prepare a statistical evaluation of rainfall and relate to history of landsliding and population of slopes within area of similar slope type.

Rainfall, and the consequent effect on groundwater levels, is widely recognized as a main trigger event for landsliding. Therefore, indicative frequency values may be related to the frequency of rainfall provided there is sufficient historical data to enable the relationship between rainfall frequency, antecedent rainfall and landslide events to be correlated.

A similar approach may be adopted for other forms of triggering events such as earthquakes.

f) Consider use of simulation models and Monte Carlo sampling analyses to derive a frequency of failure.

These methods (including simulation modelling of groundwater response to rainfall, evapotranspiration, and ground water flows) can be difficult to carry out reliably. Picarelli *et al.* (2005) outline some of the difficulties with these methods. Simulation modelling is most likely to be applicable only to medium to large, deep seated landslides where extensive monitoring data is available to enable calibration over a range of rainfall and piezometric responses.

Experience shows that full probabilistic analysis is difficult and time consuming (Robin Fell personal comm.). Therefore this method should only be carried out for special cases where sufficient data is available to enable the results to be meaningful.

g) Use knowledge based expert judgment or 'degree of belief' method which combines experience, expertise and general principles.

For most assessments this may be the only suitable option to estimate frequency due to the lack of objective data. The assessment relies to a large degree on subjective assessment of available data where other more rigorous methods are not available or viable. The method still requires some degree of research to obtain relevant data and an understanding

of the geological model to qualify the judgment of likelihood. Nonetheless, the approach requires the proposition of various possible scenarios followed by the systematic testing and elimination of options as a result of investigation, discussion and judgment to develop an estimate of frequency (Lee and Jones 2004).

The result is conditioned by the 'degree of belief' of the practitioner. Typically, the resulting accuracy for a frequency assessment and, perhaps, a consequence assessment could vary from half an order of magnitude at best, to one order of magnitude or perhaps two orders of magnitude. As a result, the risk assessment should clearly display its sensitivity to the input parameters and, unless justified by further investigations, a conservative outcome should be adopted.

h) Where appropriate, use event trees to provide a structur

i) ed and auditable approach for the use of expert judgment and subjective probability assessment.

An event tree analysis uses a graphical construct to show the logical sequence of events or considerations that can be used to analyse the system leading to a particular outcome. It can be used for evaluation of probability of failure of a landslide, or consequence of failure, or risk. The logical sequence within the system is mapped as a branching network with conditional probabilities assigned to each branch of a node. The frequency of achieving a certain outcome is the product of the conditional probabilities leading to that outcome times the frequency of the initiating "trigger" such as rainfall.

i) Other methods.

The above may not be an exhaustive list but covers the principal methods/approaches. Specific circumstances of a particular area or project may enable other approaches or combinations of approaches to be used. Field techniques may develop to offer alternatives, for example remote sensing by satellite.

Further comment is given in the Commentary together with some guidance on different site investigation methods.

5.4.2 Estimation of Annual Probability (Frequency) ($P_{(H)}$) of Each Landslide

a) Use 'best estimates' for frequency but consider range / uncertainty / sensitivity.

Suitable methods are outlined in Section 5.2.

It is important not to infer greater accuracy than is reasonably possible. Evaluation of the sensitivity arising from uncertainty is part of the consideration.

A best estimate is to be derived for each landslide which is then applied to both risk to property and risk to life assessments. The estimate may be related to the size of the landslide and/or the expected amount of movement as part of the hazard assessment. The appropriate qualitative term is chosen from the estimated probability based on the frequency assessment. Note that the reverse, the adoption of a probability value from a qualitative term, should not be undertaken as it has been demonstrated that this results in a range of estimates of frequency several orders of magnitude apart depending on the practitioner.

b) Estimates of frequency may be derived by partitioning the problem to (Annual probability of trigger event) x (Probability of sliding given the trigger event) over the range of trigger events.

Landslides of the one 'type', but having varying possible scales (magnitude/travel distance/velocity etc.) need to be assessed separately. Each could well have a different frequency of occurrence. The landslide inventory of performance for an area will provide some basis for the assessment.

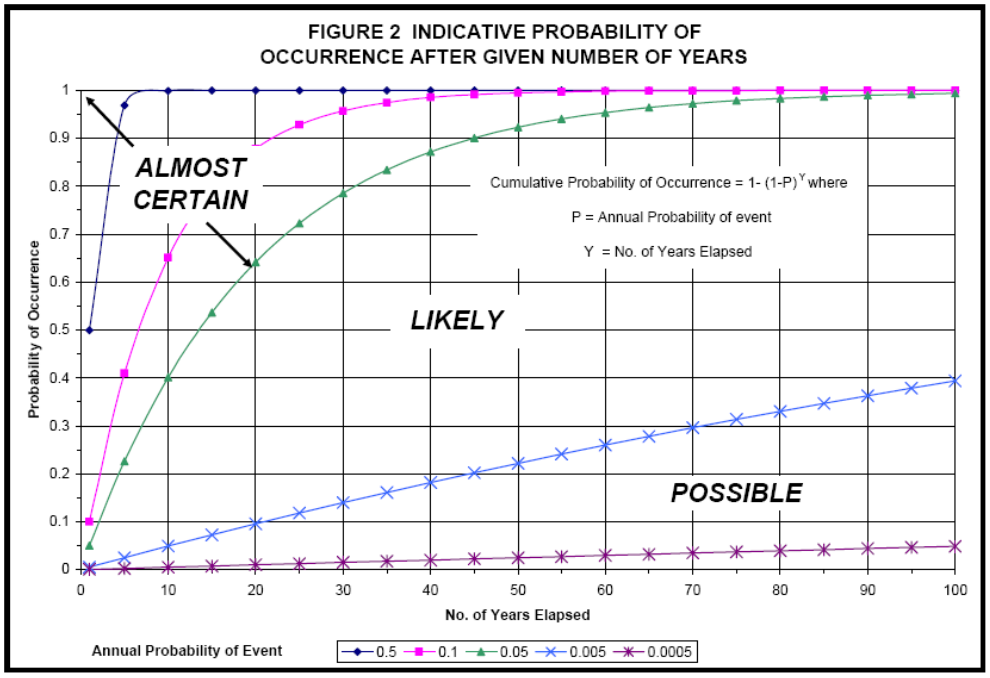
A trigger event for a particular locality (e.g. a certain intensity/duration or recurrence interval of rainfall) will not necessarily cause each potential landslide event in that locality to occur. There will be a finite probability (value) that the landslide under consideration may not be set off by the trigger event.

The frequency of landsliding should be assessed over the full range of the triggering events, and the total frequency carried forward in the risk analysis. In practice this process may be simplified to consider only the highest frequency triggering events. An example is presented in the Commentary.

c) Complete a review of the assessed frequency in relation to the implied cumulative frequency of the event occurring within the design life and known performance within the area.

This is a 'sanity check' on the result of the assessment. It is import to apply judgment or bias on the final outcome only, not on the input estimates.

Values of the cumulative probability are shown on Figure 2 for different annual probability values as a function of time over usual design life intervals. The resulting cumulative probabilities should be checked to confirm they are reasonable in relation to experience. The implications of the cumulative probability values shown in Figure 2 are discussed further in the Commentary.



5.4.3 Assess the Travel Distance and the Probability of Spatial Impact ($P_{(S:H)}$) of the Elements at Risk

When assessing risk arising from landsliding, it is important to be able to estimate the distance the slide mass will travel and its velocity. These factors determine the extent to which the landslide will affect property and persons downslope and the ability of persons to take evasive action.

The travel distance depends on:

- Slope characteristics
  - Height
  - Slope
  - Nature of material
- Mechanism of failure and type of movement such as
  - Slide, fall, topple etc.
  - Sliding, rolling, bouncing, flow
  - Strain weakening or not
  - Collapse in undrained loading (static liquefaction)
  - Influence of surface water and groundwater
  - Comminution of particles
- Characteristics of the downhill path
  - Gradient and gradient direction
  - Channelisation
  - The potential for depletion/accumulation
  - Vegetation

Information on travel distance from previous events on or near the site may be collected during the site inspection. Predictions of travel distance and travel direction should be based on the assessed mechanism of future events and site characteristics.

For rotational landslides which remain essentially intact, the method proposed by Khalili *et al* (1996) or experience with landslides in similar geological, topographic and climatic conditions can be used to estimate the displacement. Further discussion is given in the Commentary.

For slides which break up, and in some cases become flows, and slides from steep cuts, the travel distance is usually estimated from empirical methods, such as Hunter and Fell (2002) and Corominas (1996). These methods are only approximate, and the wide scatter of data on travel distance angles reflects the range of topographical, geological and climatic environments, different slide mechanisms and limited quality of data from which the methods are derived.

If the empirical methods are to be used for predictions of travel distance and the probability of spatial impact of the elements at risk, much judgement will be required and it is important to try to calibrate the methods with landslide

behaviour in the study area. It is often useful to allow for a range of travel distances in the calculation and express that range in probabilistic terms as discussed in the Commentary.

The annual probability of the landslide and probability of spatial impact may be considered together in qualitative terms as likelihood of impact on the element at risk being considered.

6 CONSEQUENCE ANALYSIS

6.1 ELEMENTS AT RISK

The elements at risk will include:

- Property, which may be subdivided into portions relative to the hazard being considered.
- People, who either live, work, or may spend some time in the area affected by landsliding.
- Services, such as water supply or drainage or electricity supply.
- Roads and communication facilities.
- Vehicles on roads, subdivided into categories (cars, trucks, buses).

These should be assessed and listed for each landslide hazard.

For some cases, other risks may also have to be considered. For example:

- Environmental, where the elements at risk are environmental (rather than man made), such as forests or water bodies.
- Social, where the consequences of the landslide may have an impact on social conditions, such as the cost of disruption to traffic where roads are affected.
- Political, where the consequences may not be acceptable in political terms.

6.2 TEMPORAL SPATIAL PROBABILITY ($P_{(T:S)}$)

When the elements at risk are mobile (e.g. persons on foot, in cars, buses and trains) or where there is varying occupancy of buildings (e.g. between night and day, week days and weekends, summer and winter), it is necessary to make allowance for the probability that persons (or a particular number of persons) will be in the area affected by the landslide. This is called the Temporal Spatial Probability**.**

For where the elements at risk are mobile it is proportion of a year (between 0 and 1.0) in which a person, car or bus will be below or on the landslide when it occurs. For occupancy of buildings it is a calculation of the proportion of a year (between 0 and 1.0) which the number of persons being considered occupy the building, or the area of the building likely to be impacted.

These calculations should allow for the possibility that the persons may have warning of trhe impending landslide and may evacuate the area. Each case should be considered by taking account of the details of the situation. Generally persons on a landslide are more likely to observe the initiation of movement and move off the slide, than those who are below a slide which falls or flows onto them unless the rates of movement are slow.

6.3 EVALUATION OF CONSEQUENCE TO PROPERTY

6.3.1 Estimate the extent of damage likely to property arising from each of the landslides.

This requires an understanding of the landslide characteristics and experience in assessing the likely impact on property. The consequences are often calculated using the vulnerability ($\mathbf{V}_{(Prop:S)}$) of the elements at risk to the landslide.

The factors which most affect vulnerability of property are:

- The volume of the slide in relation to the element at risk.
- The position of the element at risk, e.g. on the slide, or immediately downslope.
- The magnitude of slide displacement, and relative displacements within the slide (for elements sited on the slide).
- The rate of slide movement.

It should be noted that the vulnerability refers to the degree of damage (or damage value in absolute or relative terms) which is judged to be likely if the landslide does occur.

As discussed below, the assessment should be based on a quantitative estimate to enable clarification of the judgment which for a qualitative assessment may be subject to considerable interpretation.

6.3.2 Estimate the indicative cost of the damage.

This requires use of indicative costs of building and remedial works. Frequently, broad brush ‘guesstimates’ will suffice, but the ‘guesstimate values’ and basis should be documented. Some guidance is given in the Commentary. It should not be necessary to use a quantity surveyor to establish a more accurate estimate as usually the broad brush guesstimate will suffice for allocation of a consequence term in a qualitative scheme such as in Appendix C.

The indicative cost of damage is to be the Total Cost as this is the most relevant to the owner. Components to be considered comprise:-

- Direct costs related to reinstatement works for damaged portions of the property (structures and the land).
- Stabilization works required to render the site to an tolerable risk level for the landslide.
- Professional and approvals fees.
- Consequential costs (such as legal fees and alternative temporary accommodation).

It does not include additional stabilisation works to address other landslides which may affect the property.

6.3.3 Estimate the market value.

This may be achieved by reference to property sale values within the local area which will reflect the value of the land plus structures. The client is likely to have some knowledge of the local market values. Again, a broad-brush guesstimate should often suffice.

6.3.4 Consider the resulting Consequence classification, such as using Appendix C, and implied accuracy of the above estimates.

It is not expected that the assessor will be a quantity surveyor or have similar experience, but that sensible estimates, possibly as a range, can be made and documented. Statement of limits of accuracy or uncertainty are appropriate for sensitivity and appraisal analysis.

6.4 EVALUATION OF CONSEQUENCES TO PERSONS

The following factors influence the likelihood of deaths and injuries or vulnerability ($\mathbf{V}_{(D:T)}$) of persons who are impacted by a landslide:

- Volume of slide.
- Type of slide, mechanism of slide initiation and velocity of sliding.
- Depth of slide.
- Whether the landslide debris buries the person(s).
- Whether the person(s) are in the open or enclosed in a vehicle or building.
- Whether the vehicle or building collapses when impacted by debris.
- The type of collapse if the vehicle or building collapses.

Persons are very vulnerable in the event of complete or substantial burial by debris, or the collapse of a building. It should be noted that even small slides, and single boulders, can kill people.

Appendix F provides some indicative examples of vulnerability values. The Commentary provides some more detailed discussion.

7 RISK ESTIMATION

7.1 QUANTITATIVE RISK ESTIMATION

Quantitative risk estimation involves integration of the frequency analysis and the consequences.
For property, the risk can be calculated from:

$$\mathbf{R}_{(Prop)} = \mathbf{P}_{(H)} \times \mathbf{P}_{(S:H)} \times \mathbf{P}_{(T:S)} \times \mathbf{V}_{(Prop:S)} \times \mathbf{E} \quad \textbf{(1)}$$

Where

$\mathbf{R}_{(Prop)}$ is the risk (annual loss of property value).
$\mathbf{P}_{(H)}$ is the annual probability of the landslide.
$\mathbf{P}_{(S:H)}$ is the probability of spatial impact by the landslide on the property, taking into account the travel distance and travel direction.
$\mathbf{P}_{(T:S)}$ is the temporal spatial probability. For houses and other buildings $\mathbf{P}_{(T:S)}$= 1.0. For Vehicles and other moving elements at risk$1.0 < \mathbf{P}_{(T:S)} > 0$.
$\mathbf{V}_{(Prop:S)}$ is the vulnerability of the property to the spatial impact (proportion of property value lost).
$\mathbf{E}$ is the element at risk (e.g. the value or net present value of the property).

For loss of life, the individual risk can be calculated from:

$$\mathbf{R}_{(LoL)} = \mathbf{P}_{(H)} \times \mathbf{P}_{(S:H)} \times \mathbf{P}_{(T:S)} \times \mathbf{V}_{(D:T)} \quad \textbf{(2)}$$

Where

$\mathbf{R}_{(LoL)}$ is the risk (annual probability of loss of life (death) of an individual).
$\mathbf{P}_{(H)}$ is the annual probability of the landslide.
$\mathbf{P}_{(S:H)}$ is the probability of spatial impact of the landslide impacting a building (location) taking into account the travel distance and travel direction given the event.
$\mathbf{P}_{(T:S)}$ is the temporal spatial probability (e.g. of the building or location being occupied by the individual) given the spatial impact and allowing for the possibility of evacuation given there is warning of the landslide occurrence.
$\mathbf{V}_{(D:T)}$ is the vulnerability of the individual (probability of loss of life of the individual given the impact).

A full risk analysis involves consideration of all landslide hazards for the site (e.g. large, deep seated landsliding, smaller slides, boulder falls, debris flows) and all the elements at risk.

For comparison with tolerable risk criteria, the individual risk from all the landslide hazards affecting the person most at risk, or the property, should be summed.

The assessment must clearly state whether it pertains to 'as existing' conditions or following implementation of recommended risk mitigation measures, thereby giving the 'residual risk'.

7.2 SEMI-QUANTITATIVE AND QUALITATIVE RISK ESTIMATION FOR RISK TO PROPERTY

When considering the risk to property, it may be useful to use qualitative terms to report the results of the analysis, rather than quantitative values. The risk calculation may be completed quantitatively or by the use of qualitative terms.

A semi quantitative analysis (where the likelihood is linked to an indicative probability) or a qualitative analysis may be used:

- As an initial screening process to identify hazards and risks which require more detailed consideration and analysis.
- When the level of risk does not justify the time and effort required for more detailed analysis.
- Where the possibility of obtaining numerical data is limited such that a quantitative analysis is unlikely to be meaningful or may be misleading.

Section 7.3 describes a suitable and preferred terminology.

7.3 RISK MATRIX FOR PROPERTY LOSS

a) Adopt a defined qualitative terminology for likelihood, consequence and risk.

Qualitative terminology is presented in Appendix C for property loss. The terminology has been developed from Appendix G in AGS (2000) taking into account the experience and comments as discussed in the Commentary.

For ease of use, the frequency estimate, expressed as an annualized probability and taking into account the probability of spatial impact, is expressed qualitatively as likelihood.

The terminology is aimed primarily at residential development but may also be used for other situations. It is noted that provision of specific numerical values at the Notional Boundaries for the terms adopted does not reduce the uncertainty that may be associated with assessment of appropriate numerical values.

Where sufficient data is available, the risk should be determined from a quantitative analysis. The results can then be objectively compared, especially with quantified allowable risk criteria.

Where there is insufficient data or the study is at a walk over or preliminary design level, then use of qualitative methods or terms may be more appropriate. Use of risk ranking schemes, where component inputs are assigned relative ranks, may be suitable for initial screening. In other cases, it is likely that expression of the likelihood, consequence and risk using qualitative terms is preferable for communication purposes; (for example using terminology as in Appendix C). Selection of the appropriate term should be based on an appropriate evaluation of likelihood or consequence ranges.

Semi-quantitative methods may be a combination of both, for example considering risk to property qualitatively, and risk to life quantitatively based on the appropriate best estimates of likelihood.

b) The practitioner should adopt the preferred risk matrix presented in Appendix C.

The terminology presented in Appendix C of this Practice Note has addressed the shortcomings identified with the scheme in Appendix G AGS (2000). Appendix G of AGS (2000) is now superseded and should no longer be used. Adoption of Appendix C as a preferred risk matrix will assist with uniformity of assessment and interpretation. This is discussed further in the Commentary.

The regulator should only accept non standard schemes where the terms have been clearly defined, the terms have been explained in relation to the preferred terminology, and it can be reasonably demonstrated by the practitioner that the alternative is better suited to the particular circumstances of the assessment.

7.4 ESTIMATION OF RISK OF LOSS OF LIFE

a) Estimate the risk of loss of life quantitatively for the person most at risk.

The annual probability of loss of life for the person most at risk from the landslide(s) should be estimated using the equations in Section 7.1. The person most at risk will often but not always be the person with the greatest spatial temporal probability.

The individual risk, as determined by summing the risk, for the person most at risk, from all the landslide hazards, is used for comparison with the tolerable risk criteria.

b) For situations where there is a potential for large numbers of lives to be lost in a single landslide event, estimate the frequency (f) –number (N) of lives lost pairs and total annual risk.

If the possible loss of large numbers of lives from a landslide incident is high, society will generally expect that the probability that the incident might actually occur should be low. This accounts for society's particular intolerance to incidents that cause many simultaneous casualties and is embodied in the criteria for tolerable societal risk. Societal Risk is discussed further in the Commentary.

In many cases there will be more than one landslide hazard (e.g. rockfall, which may lead to one or two lives lost; medium volume rapid landslide which may lead to several lives lost; and large rapid landslide which may lead to many lives lost). The frequency (annual probability, "f") of the "event" and the number of lives lost (N) should be estimated for each landslide hazard.

The total annual risk = $\sum$ (f x N) should also be estimated.

8 RISK ASSESSMENT

8.1 RISK EVALUATION

Evaluate the risks against Tolerable Risk Criteria for loss of life and property loss.

Accept the risks if tolerable, or seek to reduce risks to tolerable levels by risk mitigation.

The main objectives of risk evaluation are usually to decide whether to accept or treat the risks and to set priorities. The Tolerable Risk Criteria are usually imposed by the regulator, unless agreed otherwise with the owner/client

Non- technical clients may seek guidance from the practitioner on whether to accept the risk. In these situations, risk comparisons, discussion of treatment options and explanation of the risk management process can help the client make his decision.

It is desirable, if not essential, that the practitioner who prepared the risk assessment be involved in the decision making process because the process is often iterative, requiring assessment of the sensitivity of calculations to assumptions, modification of the development proposed and revision of risk mitigation measures.

Risk evaluation involves making judgements about the significance and tolerability of the estimated risk. Evaluation may involve comparison of the assessed risks with other risks or with risk acceptance criteria related to finance, loss of life or other values. Risk evaluation may include consideration of issues such as environmental effects, public reaction, politics, business or public confidence and fear of litigation.

In a simple situation where the client/owner is the only affected party, risk evaluation may be a simple value judgement. In more complex situations, value judgements on acceptable risk appropriate to the particular situation are still made as part of an acceptable process of risk management.

8.2 TOLERABLE RISK CRITERIA

The regulator is to establish the Tolerable Risk Criteria for loss of life and property loss.

As discussed in Section 3.5, the regulator is the appropriate authority to set standards for tolerable risk which may relate not only to perceived safety in relation to other risks, but also to government policy. Implementation of a tolerable risk level has implications to the community at large, both in terms of relative risks or safety and in terms of economic impact on the community.

The Commentary provides discussion and gives the AGS recommendations in relation to tolerable risk for loss of life. These are summarized in Table 1

Table 1: AGS Suggested Tolerable loss of life individual risk.

Situation	Suggested Tolerable Loss of Life Risk for the person most at risk
Existing Slope (1) / Existing Development (2)	10^{-4} / annum
New Constructed Slope (3) / New Development (4) / Existing Landslide (5)	10^{-5} / annum

Notes:

1. "Existing Slopes" in this context are slopes that are not part of a recognizable landslide and have demonstrated non-failure performance over at least several seasons or events of extended adverse weather, usually being a period of at least 10 to 20 years.
2. "Existing Development" includes existing structures, and slopes that have been modified by cut and fill, that are not located on or part of a recognizable landslide and have demonstrated non-failure performance over at least several seasons or events of extended adverse weather, usually being a period of at least 10 to 20 years.
3. "New Constructed Slope" includes any change to existing slopes by cut or fill or changes to existing slopes by new stabilisation works (including replacement of existing retaining walls or replacement of existing stabilisation measures, such as rock bolts or catch fences).
4. "New Development" includes any new structure or change to an existing slope or structure. Where changes to an existing structure or slope result in any cut or fill of less than 1.0m vertical height from the toe to the crest and this change does not increase the risk, then the Existing Slope / Existing Structure criterion may be adopted. Where changes to an existing structure do not increase the building footprint or do not result in an overall change in footing loads, then the Existing Development criterion may be adopted.
5. "Existing Landslides" have been considered likely to require remedial works and hence would become a New Constructed Slope and require the lower risk. Even where remedial works are not required per se, it would be reasonable expectation of the public for a known landslide to be assessed to the lower risk category as a matter of "public safety".

Acceptable risks are usually considered to be one order of magnitude lower than the Tolerable Risks.

It is important to distinguish between "acceptable risks" and "tolerable risks".

Tolerable Risks are risks within a range that society can live with so as to secure certain benefits. It is a range of risk regarded as non-negligible and needing to be kept under review and reduced further if practicable.

Acceptable Risks are risks which everyone affected is prepared to accept. Action to further reduce such risk is usually not required unless reasonably practicable measures are available at low cost in terms of money, time and effort.

AGS suggests that for most development in existing urban area criteria based on Tolerable Risks levels are applicable because of the trade-off between the risks, the benefits of development and the cost of risk mitigation.

The Commentary discusses Individual and Societal risk to loss of life. Usually Societal risk need not be considered for a risk evaluation in relation to a single dwelling. Societal risk should be evaluated for buildings having high numbers of occupants, such as schools, hospitals, hotels or motels where many lives are at risk. This then addresses society's aversion to loss of many lives from single landslide events.

The Tolerable Risk Criteria for property loss may be determined by the Importance Level of the development (Appendix A) as discussed in the Commentary.

9 RISK MANAGEMENT

9.1 RISK MITIGATION PRINCIPLES

9.1.1 Feasible options for risk mitigation for each risk assessment are to be identified and discussed including the reduced risk by adoption of those options.

Alternative methods to be explored include:

a. ***Accept the risk***, which is only an option subject to the criteria set by the regulator. Where the risk is not tolerable then risk mitigation measures are required.

b. ***Avoid the risk***, such as relocation of the site of proposed development, or revise the form of the development, or abandon the development (though this may still require some risks to be controlled due to possible effect on third parties adjacent or nearby).

c. ***Reduce the frequency of landsliding***, by stabilisation measures to control the initiating circumstances, such as by re-profiling the surface geometry where existing slopes are 'over steep', by provision of improved surface water drainage measures, by provision of subsurface drainage scheme, by provision of retaining structures such as retaining walls, anchored walls or ground anchors.

d. ***Reduce the consequences***, by provision of defensive stabilisation measures or protective measures such as a boulder catch fence, or amelioration of the behaviour of the landslide, or by relocation of the development to a more favourable location.

e. ***Manage the risk by establishing monitoring and warning systems,*** such as by regular site visits, or by survey, which enable the risks to be managed as an interim measure in the short term or as a permanent measure for the long term by alerting persons potentially affected to a change in the landslide condition. Such systems may be regarded as a method of reducing the consequences provided it is feasible for sufficient time to be available between the alert being raised and appropriate action being implemented.

f. ***Transfer the risk,*** such as by requiring another authority to accept the risk (possibly via a court appraisal) or by provision of insurance to cover potential property damage.

g. ***Postpone the decision,*** where there is sufficient uncertainty resulting from the available data, provided that additional investigations or monitoring are likely to enable a better risk assessment to be completed. Postponement is only a temporary measure and implies the risks are being temporarily accepted, even though they may not be acceptable or tolerable.

Adoption of particular risk mitigation measures needs to be documented so that the decisions are transparent to future land owners and to the regulator. The documentation will need to make it clear whether there is ongoing maintenance required or not. Responsibility for implementation of the risk mitigation measures (including auditing and reporting) resides with the land owner, particularly where ongoing maintenance is required.

It should be recognized that there may be situations where the risk is such that either no development should occur, or that very strict conditions and/or extensive investigations and implementation of risk control measures will be required. Such risk control measures may render the proposed development unworkable.

9.1.2 Wherever possible the recommended options should be engineered to reduce the uncertainties.

It is not possible to remove risk, but it can be reduced.

Risk mitigation options should include robust engineering design to reduce uncertainties and hence the risk.

Guidance on good engineering practice for hillside design and construction is given in Appendix G which has been reproduced from AGS (2000).

It is necessary that the options considered lower the risk to at least tolerable levels. In many cases, the ALARP principle ("As Low As Reasonably Practicable" as discussed in the Commentary) may apply so that reduction to a tolerable level is a pragmatic result since reduction to acceptable levels is not viable in the context of the cost to the individual or community. In other cases, good practice may suggest that risk reduction be applied since it is relatively cheap or cost effective to implement even though risk levels are assessed to already be at acceptable levels. In other words, risk minimization should be a governing feature or tenet of LRM.

Evaluation of mitigation options may take into account relative costs and effectiveness of the measures and inherent uncertainties. Combinations of mitigation measures may be appropriate.

The options should be reassessed if there is a need to reduce uncertainties or if suitable engineering options cannot be adopted.

An issue will be who decides on what level of risk reduction is appropriate. This is dependent on the risk tolerance criteria set by the regulator. The owner is likely to input into selection of the options, subject to approvals by the regulator. For some cases, there may be discussion between the stakeholders to select a suitable scheme of risk mitigation measures.

9.1.3 The adopted risk mitigation measures are to be detailed in a mitigation plan to explain and document the implementation of the measures.

The mitigation plan should identify responsibilities for each stakeholder during and after implementation. It may also include cost estimates, programme, required inspection regime, performance measures and expected outcomes. The level of detail will depend on the priority for the option and stage of the evaluation and implementation process.

The mitigation plan may include an emergency plan which should establish from the outset the sequence of events or monitoring results that will activate this plan. The plan may include a number of warning levels and consequent actions. The plan must be carefully reviewed to confirm it is workable and will achieve the desired risk mitigation.

The existence of the mitigation plan needs to be readily known to subsequent land owners. The most readily available method for this is to register the mitigation plan details on the land title.

9.1.4 The risk should be subject to monitoring and review during the assessment of options, during implementation of the risk mitigation measures and during the on going monitoring.

Further data may come to light during the management process which enables the risks to be reassessed. Such data may be adverse, requiring more stringent risk mitigation measures, or alternatively may be positive by demonstrating satisfactory slope performance under adverse conditions. It is anticipated that the practitioner would have a primary role in the monitoring and review process and particularly to confirm the requirements of the approval conditions had been fulfilled.

9.2 SITE SPECIFIC DEVELOPMENT CONDITIONS

Identify appropriate site specific development conditions to provide good practice and control the risks to acceptable levels.

In the context of advice from a technical expert (the practitioner) acting in a consultant capacity, development controls would usually constitute 'recommendations', but as they will be integral with the risk assessment of the final development they may not be optional to the client. The practitioner should provide a statement as to the appropriateness of the development proposals in relation to the risk management requirements.

If 'certification' of the completed development is required (by the planning scheme or regulator's approval conditions), then the development conditions and associated inspections and documentation must be sufficient to enable this to be provided at the later date.

The development conditions should be subdivided into those required at each of the stages of detailed design, construction (including appropriate sequencing and temporary works), and for maintenance. The development conditions must address all the factors relevant to controlling the landslide risk.

9.3 DESIGN LIFE

9.3.1 Design of the risk mitigation measures is to be suitable for the time frame of the life of the structure - the design life. The design life is to be clearly stated on the design drawings.

Often the design life will be that specified by relevant design codes such as 40 to 60 years for AS3600 Concrete Code, 50 years for AS2870 Residential Slabs and Footings, or for 5 years to 120 years for temporary site works to major public works respectively for AS4678 Earth Retaining Structures.

A design life of at least 50 years would be considered to be reasonable for permanent structures used by people. Some local government policies may require a longer design life as discussed in the Commentary. However, for some structures, such as timber retaining walls, inherent performance of the materials will limit the effective performance life to less than the required design life.

9.3.2 Where the effective performance life is less than the required design life, then the effective life should be extended by a maintenance regime designed to overcome the limitations and to enable the performance to be assessed throughout the required design life. This is likely to require more extensive repair and replacement as determined by regular maintenance inspections.

For example, experience shows the longevity of timber crib walls is less than for a concrete structure, due to faster degradation of timber with time. Therefore, a more frequent inspection and maintenance / repair / replacement regime will be required for timber crib walls to enable suitable repair and replacement so that a reasonable design life can be achieved. Similar considerations will apply to subsoil drains and stressed anchors.

9.4 MAINTENANCE REQUIREMENTS

9.4.1 The design is to include details of required inspections and maintenance to enable the risk mitigation measures to remain effective for at least the design life of the structure.

Risk mitigation is not just an exercise in LRM documentation, design of the works and construction of the risk mitigation measures. The owner, including all owners subsequent to those responsible for commissioning the risk mitigation measures, has a responsibility to inspect and maintain the risk mitigation measures.

9.4.2 Refer to the AGS Australian GeoGuide LR111 which provides advice on record keeping.

The other GeoGuides (AGS, 2007e) also provide advice on the frequency of maintenance tasks.

9.4.3 Implementation of the maintenance plan may require 'enforcement' by annotation on the land title so that subsequent purchasers become aware of the requirements and that relevant documents are available for the maintenance plan. Such 'enforcement' will be a benefit to subsequent owners as they will be better informed as to their required input responsibilities.

10 REPORTING STANDARDS

10.1 The report on the risk assessment is to document the data gathered, the logic applied and conclusion reached in a defensible manner.

The practitioner will gather relevant data, will assess the relevance of the data and will reach conclusions as to the appropriate geotechnical model and basic assessment of the slope forming processes and rates. Full documentation of these results provides evidence of completion, provides transparency in the light of uncertainty, enables the assessment to be re-examined or extended at a later date and enables the assessment to be defended against critical review. The process often identifies uncertainties or limitations of the assessment which also need to be documented and understood.

10.2 The data to be presented includes:

a. List of data sources.
b. Discussion of investigation methods used, and any limitations thereof.
c. Site plan (to scale) with geomorphic mapping results.
d. All factual data from investigations, such as borehole and test pit logs, laboratory test results, groundwater level observations, record photographs.
e. Location of all subsurface investigations and/or outcrops/cuttings.
f. Location of cross section(s).
g. Cross section(s) (to scale) with interpreted subsurface model showing investigation locations.
h. Evidence of past performance.
i. Local history of instability with assessed trigger events.
j. Identification of landslides, on plan or section or both, and discussed in terms of the geomorphic model, relevant slope forming process and process rates. Landslides need to be considered above the site, below the site and adjacent to the site.
k. Assessed likelihood of each landslide with basis thereof.
l. Assessed consequence to property and life for each landslide with basis thereof.
m. Resulting risk for each landslide.
n. Risk assessment in relation to tolerable risk criteria (e.g. regulator's published criteria where appropriate).
o. Risk mitigation measures and options, including reassessed risk once these measures are implemented.

Where any of the above is not or cannot be completed, the report should document the missing elements, including an explanation as to why.

The report needs to clearly state whether the risk assessment is based on existing conditions or with risk treatment measures implemented. In some cases, the assessment for both existing and after treatment should be documented to demonstrate the effect of risk control measures on reducing risk.

A report which does not properly document the assessment is of limited value and would appear to have no reasonable basis.

11 SPECIAL CHALLENGES

11.1 MINOR WORKS

Adoption of all the provisions of the Practice Note for minor works may not be appropriate or reasonable. However, the basic principles still need to be considered. Although some policies may make provision for less onerous consideration for minor works, the practitioner will still have a duty of care to advise on all aspects and may have other landslides not connected with the proposed works that will still need to be considered.

Minor works should be evaluated on a site by site basis but are likely to comprise proposed works of relatively low monetary value (such as may be completed by an owner builder with appropriate approvals and insurances) or those which do not change the existing risk, provided the existing risk has been assessed to be within the tolerable range. In some cases, the risk to life may be much higher than the risk to property and may dictate the need for risk mitigation to achieve tolerable risk levels.

11.2 PART OF THE SITE NOT ACCEPTABLE

Existing or proposed development may not involve the full site area. Nonetheless, the practitioner's report must address all risks and advise the client and/or regulator of necessary works to control risks on other parts of the site or adjacent/nearby sites upslope or down slope as appropriate (as a primary duty of care issue).

Where additional development is proposed, it may be found that risks associated with the proposed development are tolerable but that landslide risks on other parts of the site are not. These other risks still must be addressed.

11.3 ADJOINING AREAS NOT UNDER RESPONSIBILITY OF THE SITE OWNER

In some cases, the risk posed by landslides in areas beyond the control of the land owner may be intolerable.

The LRM assessment report must identify these landslides and provide a preliminary assessment of appropriate risk mitigation measures, which may require further investigation to better assess the risk.

The regulator may then implement appropriate orders (as appropriate to the legal/regulatory framework) to enforce appropriate risk mitigation measures and/or investigations. Alternatively, it may not be appropriate for development to proceed in such cases.

11.4 COASTAL CLIFFS

LRM reports on coastal cliffs should include consideration of the existing slope profile, evidence of past instability, geology, defects, ground water, degradation cycles, and degradation rates and possible effects of wave attack, wave run-up and sea spray. The cliff areas should be examined from the face side as well as from the land side.

Assessment of coastal cliffs is likely to require special expertise to consider the combined effects associated with recession rates, rock mechanics and wave environment. The LRM assessment may require some input from coastal engineers to address possible effects from storm events in terms of wave heights, run-up and frequency. The most frequent hazard is often boulder falls which will have risk determined by the temporal spatial probability.

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APPENDIX A - DEFINITION OF TERMS AND LANDSLIDE RISK

RISK TERMINOLOGY

Acceptable Risk – A risk for which, for the purposes of life or work, we are prepared to accept as it is with no regard to its management. Society does not generally consider expenditure in further reducing such risks justifiable.

Annual Exceedance Probability (AEP) – The estimated probability that an event of specified magnitude will be exceeded in any year.

Consequence – The outcomes or potential outcomes arising from the occurrence of a landslide expressed qualitatively or quantitatively, in terms of loss, disadvantage or gain, damage, injury or loss of life.

Elements at Risk – The population, buildings and engineering works, economic activities, public services utilities, infrastructure and environmental features in the area potentially affected by landslides.

Frequency – A measure of likelihood expressed as the number of occurrences of an event in a given time. See also Likelihood and Probability.

Hazard – A condition with the potential for causing an undesirable consequence (the landslide). The description of landslide hazard should include the location, volume (or area), classification and velocity of the potential landslides and any resultant detached material, and the likelihood of their occurrence within a given period of time.

Individual Risk to Life – The risk of fatality or injury to any identifiable (named) individual who lives within the zone impacted by the landslide; or who follows a particular pattern of life that might subject him or her to the consequences of the landslide.

Landslide Activity – The stage of development of a landslide; pre failure when the slope is strained throughout but is essentially intact; failure characterised by the formation of a continuous surface of rupture; post failure which includes movement from just after failure to when it essentially stops; and reactivation when the slope slides along one or several pre-existing surfaces of rupture. Reactivation may be occasional (eg seasonal) or continuous (in which case the slide is "active").

Landslide Intensity – A set of spatially distributed parameters related to the destructive power of a landslide. The parameters may be described quantitatively or qualitatively and may include maximum movement velocity, total displacement, differential displacement, depth of the moving mass, peak discharge per unit width, kinetic energy per unit area.

Landslide Risk - The AGS Australian GeoGuide LR7 (AGS, 2007e) should be referred to for an explanation of Landslide Risk.

Landslide Susceptibility – The classification, and volume (or area) of landslides which exist or potentially may occur in an area or may travel or retrogress onto it. Susceptibility may also include a description of the velocity and intensity of the existing or potential landsliding.

Likelihood – Used as a qualitative description of probability or frequency.

Probability – A measure of the degree of certainty. This measure has a value between zero (impossibility) and 1.0 (certainty). It is an estimate of the likelihood of the magnitude of the uncertain quantity, or the likelihood of the occurrence of the uncertain future event.

There are two main interpretations:

(i) Statistical – frequency or fraction – The outcome of a repetitive experiment of some kind like flipping coins. It includes also the idea of population variability. Such a number is called an "objective" or relative frequentist probability because it exists in the real world and is in principle measurable by doing the experiment.

(ii) Subjective probability (degree of belief) – Quantified measure of belief, judgment, or confidence in the likelihood of an outcome, obtained by considering all available information honestly, fairly, and with a minimum of

bias. Subjective probability is affected by the state of understanding of a process, judgment regarding an evaluation, or the quality and quantity of information. It may change over time as the state of knowledge changes.

Qualitative Risk Analysis – An analysis which uses word form, descriptive or numeric rating scales to describe the magnitude of potential consequences and the likelihood that those consequences will occur.

Quantitative Risk Analysis – An analysis based on numerical values of the probability, vulnerability and consequences and resulting in a numerical value of the risk.

Risk – A measure of the probability and severity of an adverse effect to health, property or the environment. Risk is often estimated by the product of probability x consequences. However, a more general interpretation of risk involves a comparison of the probability and consequences in a non-product form.

Risk Analysis – The use of available information to estimate the risk to individual, population, property, or the environment, from hazards. Risk analyses generally contain the following steps: Scope definition, hazard identification and risk estimation.

Risk Assessment – The process of risk analysis and risk evaluation.

Risk Control or **Risk Treatment** – The process of decision making for managing risk and the implementation or enforcement of risk mitigation measures and the re-evaluation of its effectiveness from time to time, using the results of risk assessment as one input.

Risk Estimation – The process used to produce a measure of the level of health, property or environmental risks being analysed. Risk estimation contains the following steps: frequency analysis, consequence analysis and their integration.

Risk Evaluation – The stage at which values and judgments enter the decision process, explicitly or implicitly, by including consideration of the importance of the estimated risks and the associated social, environmental and economic consequences, in order to identify a range of alternatives for managing the risks.

Risk Management – The complete process of risk assessment and risk control (or risk treatment).

Societal Risk – The risk of multiple fatalities or injuries in society as a whole: one where society would have to carry the burden of a landslide causing a number of deaths, injuries, financial, environmental and other losses.

Susceptibility – see Landslide Susceptibility

Temporal Spatial Probability – The probability that the element at risk is in the area affected by the landsliding, at the time of the landslide.

Tolerable Risk – A risk within a range that society can live with so as to secure certain net benefits. It is a range of risk regarded as non-negligible and needing to be kept under review and reduced further if possible.

Vulnerability – The degree of loss to a given element or set of elements within the area affected by the landslide hazard. It is expressed on a scale of 0 (no loss) to 1 (total loss). For property, the loss will be the value of the damage relative to the value of the property; for persons, it will be the probability that a particular life (the element at risk) will be lost, given the person(s) is affected by the landslide.

ASSOCIATED TERMINOLOGY

Importance Level – of a building or structure is directly related to the societal requirements for its use, particularly during or following extreme events. The consequences with respect to life safety of the occupants of buildings are indirectly related to the Importance Level, being a result of the societal requirement for the structure rather than the reason *per se* of the Importance Level.

Authority or **Council** having statutory responsibility for community activities, community safety and development approval or management of development within its defined area/region.

The **Regulator** will be the responsible body/authority for setting Acceptable/Tolerable Risk Criteria to be adopted for the community/region/activity, which will be the basis for setting levels for Acceptable and Tolerable Risk in the application of the risk assessment guidelines.

Importance Level of Structure	Explanation	Examples (Regulatory authorities may designate any structure to any classification type when local conditions make such desirable)
1	Buildings or structures generally presenting a low risk to life and property (including other property).	Farm buildings. Isolated minor storage facilities. Minor temporary facilities. Towers in rural situations.
2	Buildings and structures not covered by Importance Levels 1, 3 or 4.	Low-rise residential construction. Buildings and facilities below the limits set for Importance Level 3.
3	Buildings or structures that as a whole may contain people in crowds, or contents of high value to the community, or that pose hazards to people in crowds.	Buildings and facilities where more than 300 people can congregate in one area. Buildings and facilities with primary school, secondary school or day-care facilities with capacity greater than 250. Buildings and facilities for colleges or adult education facilities with a capacity greater than 500. Health care facilities with a capacity of 50 or more residents but no having surgery or emergency treatment facilities. Jails and detention facilities. Any occupancy with an occupant load greater than 5,000. Power generating facilities, water treatment and waste water treatment facilities, any other public utilities not included in Importance Level 4. Buildings and facilities not included in Importance Level 4 containing hazardous materials capable of causing hazardous conditions that do not extend beyond property boundaries.
4	Buildings or structures that are essential to post-disaster recovery, or with significant post-disaster functions, or that contain hazardous materials.	Buildings and facilities designated as essential facilities. Buildings and facilities with special post-disaster functions. Medical emergency or surgery facilities. Emergency service facilities: fire, rescue, police station and emergency vehicle garages. Utilities required as back-up for buildings and facilities of Importance Level 4. Designated emergency shelters. Designated emergency centres and ancillary facilities. Buildings and facilities containing hazardous (toxic or explosive) materials in sufficient quantities capable of causing hazardous conditions that extend beyond property boundaries.

(from BCA Guidelines)

Practitioner – A specialist Geotechnical Engineer or Engineering Geologist who is degree qualified, is a member of a professional institute and who has achieved chartered professional status – being either Chartered Professional Engineer (CPEng) within the Institution of Engineers Australia, Chartered Professional Geologist (CPGeo) within the Australasian Institute of Mining & Metallurgy, or Registered Professional Geoscientist (RPGeo) within the Australian Institute of Geoscientists – specifically with Landslide Risk Management as a core competency.

A Practitioner will include persons qualified under the Institution of Engineers Australia NPER – LRM register.

It would normally be required that the Practitioner can demonstrate an appropriate minimum period of experience in the practice of landslide risk assessment and management in the geographic region, or can demonstrate relevant experience in similar geological settings.

Regulator – The regulatory authority [Federal Government/ State Government/ Instrumentality/ Regional/Local.

APPENDIX B - LANDSLIDE TERMINOLOGY

The following provides a summary of landslide terminology which should (for uniformity of practice) be adopted when classifying and describing a landslide. It has been based on Cruden & Varnes (1996) and the reader is recommended to refer to the original documents for a more detailed discussion, other terminology and further examples of landslide types and processes.

Landslide

The term ***landslide*** denotes "the movement of a mass of rock, debris or earth down a slope". The phenomena described as landslides are not limited to either the "land" or to "sliding", and usage of the word has implied a much more extensive meaning than its component parts suggest. Ground subsidence and collapse are excluded.

Classification of Landslides

Landslide classification is based on Varnes (1978) system which has two terms: the first term describes the material type and the second term describes the type of movement.

The material types are ***Rock***, ***Earth*** and ***Debris***, being classified as follows:-

The material is either rock or soil.

Rock: is "a hard or firm mass that was intact and in its natural place before the initiation of movement."

Soil: is "an aggregate of solid particles, generally of minerals and rocks, that either was transported or was formed by the weathering of rock in place. Gases or liquids filling the pores of the soil form part of the soil."

Earth: "describes material in which 80% or more of the particles are smaller than 2 mm, the upper limit of sand sized particles."

Debris: "contains a significant proportion of coarse material; 20% to 80% of the particles are larger than 2 mm and the remainder are less than 2 mm."

The terms used should describe the displaced material in the landslide before it was displaced.

The types of movement describe how the landslide movement is distributed through the displaced mass. The five kinematically distinct types of movement are described in the sequence ***fall***, ***topple***, ***slide***, ***spread*** and ***flow***.

The following table shows how the two terms are combined to give the landslide type:

Table B1: Major types of landslides. Abbreviated version of Varnes' classification of slope movements (Varnes, 1978).

TYPE OF MOVEMENT		TYPE OF MATERIAL		
		BEDROCK	ENGINEERING SOILS	
			Predominantly Coarse	Predominantly Fine
FALLS		Rock fall	Debris fall	Earth fall
TOPPLES		Rock topple	Debris topple	Earth topple
SLIDES	ROTATIONAL	Rock slide	Debris slide	Earth slide
	TRANSLATIONAL			
LATERAL SPREADS		Rock spread	Debris spread	Earth spread
FLOWS		Rock flow (Deep creep)	Debris flow (Soil creep)	Earth flow
COMPLEX	Combination of two or more principle types of movement			

Figure B1 gives schematics to illustrate the major types of landslide movement. Further information and photographs of landslides are available on the USGS website at http://landslides.usgs.gov.

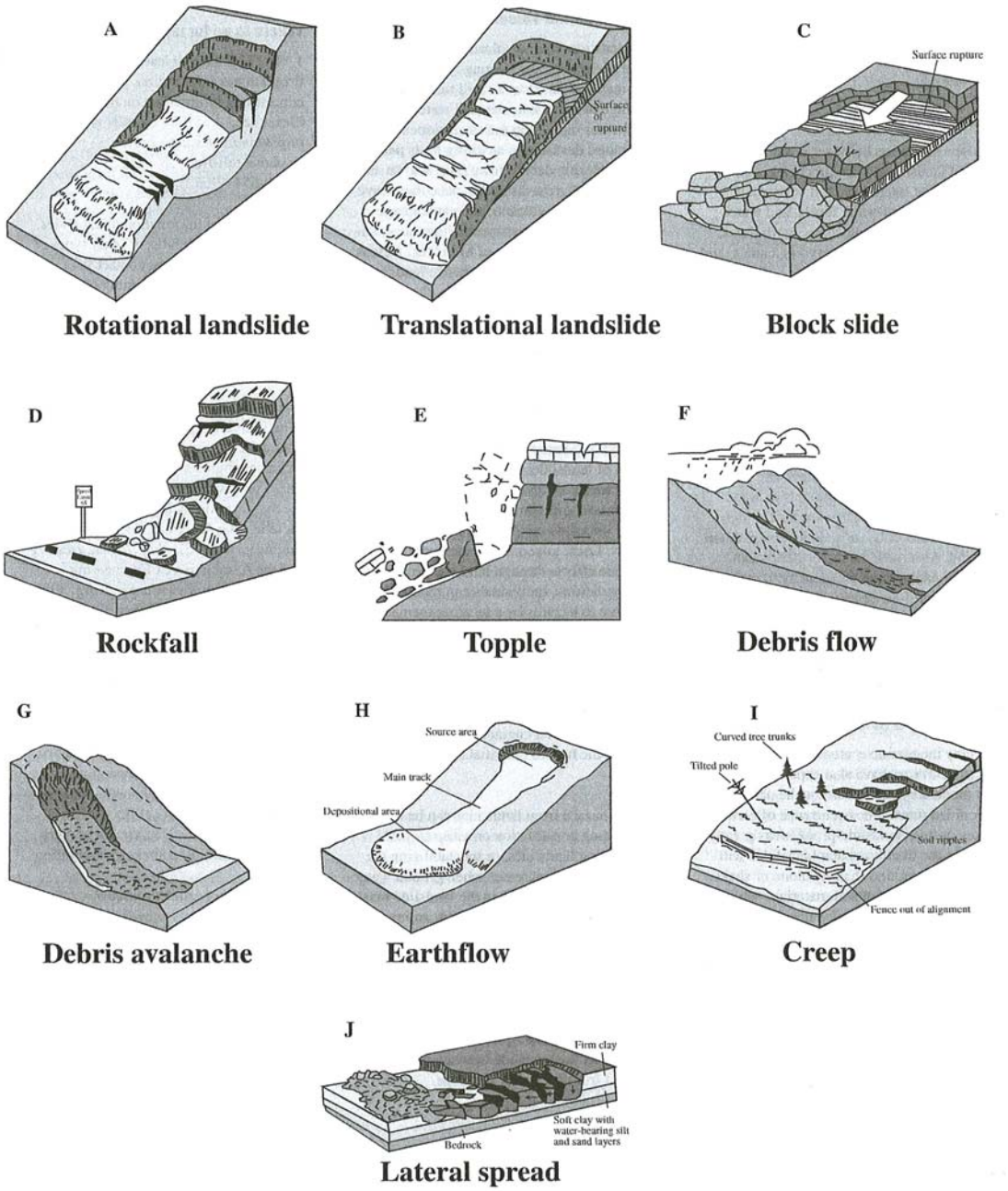

Figure B1: These schematics illustrate the major types of landslide movement.
(From US Geological Survey Fact Sheet 2004-3072, July 2004, with kind permission for reproduction.)

The nomenclature of a landslide can become more elaborate as more information about the movement becomes available. To build up the complete identification of the movement, descriptors are added in front of the two-term classification using a preferred sequence of terms. The suggested sequence provides a progressive narrowing of the focus of the descriptors, first by time and then by spatial location, beginning with a view of the whole landslide, continuing with parts of the movement and finally defining the materials involved. The recommended sequence, as shown in Table B2, describes activity (including state, distribution and style) followed by descriptions of all movements (including rate, water content, material and type). Definitions of the terms in Table B2 are given in Cruden & Varnes (1996).

Second or subsequent movements in complex or composite landslides can be described by repeating, as many times as necessary, the descriptors used in Table B2. Descriptors that are the same as those for the first movement may then be dropped from the name.

For example, the very large and rapid slope movement that occurred near the town of Frank, Alberta, Canada, in 1903 was a *complex, extremely rapid, dry rock fall – debris flow*. From the full name of this landslide at Frank, one would know that both the debris flow and the rock fall were extremely rapid and dry because no other descriptors are used for the debris flow.

The full name of the landslide need only be given once; subsequent references should then be to the initial material and type of movement; for the above example, "the rock fall" or "the Frank rock fall" for the landslide at Frank, Alberta.

Table B2: Glossary for forming names of landslides.

Activity

State	Distribution	Style
Active	Advancing	Complex
Reactivated	Retrogressive	Composite
Suspended	Widening	Multiple
Inactive	Enlarging	Successive
Dormant	Confined	Single
Abandoned	Diminishing	
Stabilised	Moving	
Relict		

Description of First Movement

Rate	Water Content	Material	Type
Extremely rapid	Dry	Rock	Fall
Very rapid	Moist	Earth	Topple
Rapid	Wet	Debris	Slide
Moderate	Very Wet		Spread
Slow			Flow
Very slow			
Extremely slow			

Note: Subsequent movements may be described by repeating the above descriptors as many times as necessary. These terms are described in more detail in Cruden & Varnes (1996) and examples are given.

Landslide Features

Varnes (1978, Figure 2.1t) provided an idealised diagram showing the features for a *complex earth slide – earth flow*, which has been reproduced here as Figure B2. Definitions of landslide dimensions are given in Cruden & Varnes (1996).

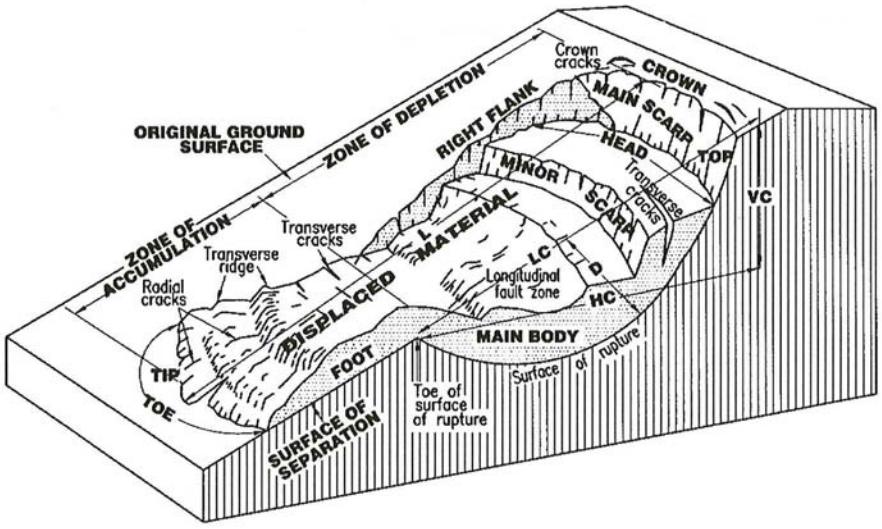

Figure B2: Block of Idealised Complex Earth Slide – Earth Flow

(*Varnes, D J (1978,)Slope Movement Types and Processes. In Special Report 176: Landslides: Analysis and Control(R L Schuster & R J Krizek, eds.), TRB, National Research Council, Washington, DC, pp.11-33).*

Rate of Movement

Figure B3 shows the velocity scale proposed by Cruden & Varnes (1996) which rationalises previous scales. The term "creep" has been omitted due to the many definitions and interpretations in the literature.

Velocity Class	Description	Velocity (mm/sec)	Typical Velocity	Probable Destructive Significance
7	Extremely Rapid			Catastrophe of major violence; buildings destroyed by impact of displaced material; many deaths; escape unlikely
		5×10^{3}	5 m/sec	
6	Very Rapid			Some lives lost; velocity too great to permit all persons to escape
		5×10^{1}	3 m/min	
5	Rapid			Escape evaluation possible; structures; possessions, and equipment destroyed
		5×10^{-1}	1.8 m/hr	
4	Moderate			Some temporary and insensitive structures can be temporarily maintained
		5×10^{-3}	13 m/month	
3	Slow			Remedial construction can be undertaken during movement; insensitive structures can be maintained with frequent maintenance work if total movement is not large during a particular acceleration phase
		5×10^{-5}	1.6 m/year	
2	Very Slow			Some permanent structures undamaged by movement
		5×10^{-7}	15 mm/year	
	Extremely **SLOW**			Imperceptible without instruments; construction **POSSIBLE WITH PRECAUTIONS**

Figure B3: Proposed Landslide Velocity Scale and Probable Destructive Significance.

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PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

APPENDIX C: LANDSLIDE RISK ASSESSMENT

QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY

QUALITATIVE MEASURES OF LIKELIHOOD

Approximate Annual Probability		Implied Indicative Landslide Recurrence Interval		Description	Descriptor	Level
Indicative Value	Notional Boundary					
10^{-1}	$5x10^{-2}$	10 years	20 years	The event is expected to occur over the design life.	ALMOST CERTAIN	A
10^{-2}	$5x10^{-3}$	100 years	200 years	The event will probably occur under adverse conditions over the design life.	LIKELY	B
10^{-3}	$5x10^{-4}$	1000 years	2000 years	The event could occur under adverse conditions over the design life.	POSSIBLE	C
10^{-4}	$5x10^{-5}$	10,000 years	20,000 years	The event might occur under very adverse circumstances over the design life.	UNLIKELY	D
10^{-5}	$5x10^{-6}$	100,000 years	200,000 years	The event is conceivable but only under exceptional circumstances over the design life.	RARE	E
10^{-6}		1,000,000 years		The event is inconceivable or fanciful over the design life.	BARELY CREDIBLE	F

Note: (1) The table should be used from left to right; use Approximate Annual Probability or Description to assign Descriptor, not *vice versa*.

QUALITATIVE MEASURES OF CONSEQUENCES TO PROPERTY

Approximate Cost of Damage		Description	Descriptor	Level
Indicative Value	Notional Boundary			
200%	100%	Structure(s) completely destroyed and/or large scale damage requiring major engineering works for stabilisation. Could cause at least one adjacent property major consequence damage.	CATASTROPHIC	1
60%	40%	Extensive damage to most of structure, and/or extending beyond site boundaries requiring significant stabilisation works. Could cause at least one adjacent property medium consequence damage.	MAJOR	2
20%	10%	Moderate damage to some of structure, and/or significant part of site requiring large stabilisation works. Could cause at least one adjacent property minor consequence damage.	MEDIUM	3
5%	1%	Limited damage to part of structure, and/or part of site requiring some reinstatement stabilisation works.	MINOR	4
0.5%		Little damage. (Note for high probability event (Almost Certain), this category may be subdivided at a notional boundary of 0.1%. See Risk Matrix.)	INSIGNIFICANT	5

Notes: (2) The Approximate Cost of Damage is expressed as a percentage of market value, being the cost of the improved value of the unaffected property which includes the land plus the unaffected structures.

(3) The Approximate Cost is to be an estimate of the direct cost of the damage, such as the cost of reinstatement of the damaged portion of the property (land plus structures), stabilisation works required to render the site to tolerable risk level for the landslide which has occurred and professional design fees, and consequential costs such as legal fees, temporary accommodation. It does not include additional stabilisation works to address other landslides which may affect the property.

(4) The table should be used from left to right; use Approximate Cost of Damage or Description to assign Descriptor, not vice versa

PRACTICE NOTE GUIDELINES FOR LANDSLIDE RISK MANAGEMENT 2007

APPENDIX C: – QUALITATIVE TERMINOLOGY FOR USE IN ASSESSING RISK TO PROPERTY (CONTINUED)

QUALITATIVE RISK ANALYSIS MATRIX – LEVEL OF RISK TO PROPERTY

LIKELIHOOD		CONSEQUENCES TO PROPERTY (With Indicative Approximate Cost of Damage)				
	Indicative Value of Approximate Annual Probability	**1: CATASTROPHIC 200%**	**2: MAJOR 60%**	**3: MEDIUM 20%**	**4: MINOR 5%**	**5: INSIGNIFICANT 0.5%**
A – ALMOST CERTAIN	10^{-1}	VH	VH	VH	H	M or **L** (5)
B - LIKELY	10^{-2}	VH	VH	H	M	**L**
C - POSSIBLE	10^{-3}	VH	H	M	M	**VL**
D - UNLIKELY	10^{-4}	H	M	**L**	**L**	**VL**
E - RARE	10^{-5}	M	**L**	**L**	**VL**	**VL**
F - BARELY CREDIBLE	10^{-6}	**L**	**VL**	**VL**	**VL**	**VL**

Notes: (5) For Cell A5, may be subdivided such that a consequence of less than 0.1% is Low Risk.
(6) When considering a risk assessment it must be clearly stated whether it is for existing conditions or with risk control measures which may not be implemented at the current time.

RISK LEVEL IMPLICATIONS

Risk Level		**Example Implications** (7)
VH	VERY HIGH RISK	Unacceptable without treatment. Extensive detailed investigation and research, planning and implementation of treatment options essential to reduce risk to Low; may be too expensive and not practical. Work likely to cost more than value of the property.
H	HIGH RISK	Unacceptable without treatment. Detailed investigation, planning and implementation of treatment options required to reduce risk to Low. Work would cost a substantial sum in relation to the value of the property.
M	MODERATE RISK	May be tolerated in certain circumstances (subject to regulator's approval) but requires investigation, planning and implementation of treatment options to reduce the risk to Low. Treatment options to reduce to Low risk should be implemented as soon as practicable.
L	LOW RISK	Usually acceptable to regulators. Where treatment has been required to reduce the risk to this level, ongoing maintenance is required.
VL	VERY LOW RISK	Acceptable. Manage by normal slope maintenance procedures.

Note: (7) The implications for a particular situation are to be determined by all parties to the risk assessment and may depend on the nature of the property at risk; these are only given as a general guide.

APPENDIX D -EXAMPLE FORMS

The following example forms have been prepared as templates to provide appropriate documentation for the control of submissions and approval process.

It is envisaged that the regulator would edit the forms to suit local requirements and to use terminology appropriate to regulatory framework of the regulator's LRM policy. Items between '< >' are to be edited as appropriate. The following terms have been used in a generic sense and should be amended by the regulator accordingly:

<the Regulator> - the authority responsible for the approval of the development application.

<Regulator's geotechnical DCP> - the appropriate LRM policy title/reference, or Development Control Plan (DCP).

<add reference> - the section or page of the geotechnical report which addresses the item.

<PCA> - the Principal Certifying Authority, or the authority who will be responsible for confirmation of compliance with the development approval conditions.

<tolerable risk> - amend to 'acceptable risk' if that is required by the <Regulator's geotechnical DCP> rather than tolerable.

<Construction Certificate> - the approval necessary to start construction which documents that design has complied with the conditions of approval for the development application.

<Occupation Certificate> - the final approval from the Regulator allowing occupation of the development once all required conditions of consent have been shown to be satisfied.

<Subdivision Certificate> - the final approval from the Regulator confirming that subdivision works have been completed in accordance with the conditions of consent such that development on individual lots may proceed.

<Building Certificate> - a certificate issued by the Regulator confirming that either existing development is in accordance with the Regulator's requirements, or confirming that the Regulator is not aware of any non-compliance which will require rectification works.

ACKNOWLEDGEMENT

These example forms have been based on the forms included in the Wollongong City "Geotechnical Development Control Plan - Development of Sites which may be subject to Slope Instability", effective from 12 July 2006 - with their kind permission. Copies of the Word documents may be obtained from AGS by regulators wishing to prepare their own forms.

Page 1 of 2

FORM A

Geotechnical Declaration and Verification Development Application

Office Use Only		Regulator: <*Add in or change to appropriate name*>

To be submitted with a development application. If this form is not submitted with the geotechnical report the report will be refused.
This form is essential to verify that the geotechnical report has been prepared in accordance with <*Regulator's geotechnical DCP*> and that the author of the geotechnical report is a geotechnical engineer or engineering geologist as defined by <*Regulator's geotechnical DCP*>. Alternatively, where a geotechnical report has been prepared for subdivision or is greater than two years old or by a professional person not recognised by <*Regulator's geotechnical DCP*> , then this form may be used as technical verification of the geotechnical report if signed by a geotechnical engineer or engineering geologist as defined by <*Regulator's geotechnical DCP*>.

Section 1 Related Application

Reference	What is the Council development application number?
DA Site Address	
DA Applicant	

Section 2 Geotechnical Report

Details	Title:	
	Author's Company/ Organisation Name:	Report Reference No:
	Author:	Dated: / /

Section 3 Checklist

Geotechnical Requirements (Tick as appropriate, either Yes or No)

The following checklist covers the minimum requirements to be addressed in a geotechnical report. This checklist is to accompany the report. Each item is to be cross-referenced to the section or page of the geotechnical report which addresses that item.

Yes	No	
☐	☐	A review of readily available history of slope instability in the site or related land as per <*Add reference*>
☐	☐	An assessment of the risk posed by all reasonably identifiable geotechnical hazards as per <*Add reference*>
☐	☐	Plans and sections of the site and related land as per <*Add reference*>
☐	☐	Presentation of a geological model as per <*Add reference*>
☐	☐	Photographs and/or drawings of the site as per <*Add reference*>
☐	☐	A conclusion as to whether the site is suitable for the development proposed to be carried out either conditionally or unconditionally as per <*Add reference*>
☐	☐	If any items above are ticked No, an explanation is to be included in the report to justify why. <*Add reference*>

Subject to recommendations and conditions relevant to:

Yes	No	
☐	☐	selection and construction of footing systems,
☐	☐	earthworks,
☐	☐	surface and sub surface drainage,
☐	☐	recommendations for the selection of structural systems consistent with the geotechnical assessment of the risk,
☐	☐	any conditions that may be required for the ongoing mitigation and maintenance of the site and the proposal, from a geotechnical viewpoint,
☐	☐	highlighting and detailing the inspection regime to provide the <PCA> and builder with adequate notification for all necessary inspections.
☐	☐	State Design life adopted: Years

Note: <Add reference>: Add in the relevant section or page number of the listed geotechnical report which addresses each item.

FORM A

Geotechnical Declaration and Verification Development Application

Section 4 List of Drawings referenced in Geotechnical Report

Design Documents	Description	Plan or Document No.	Revision or Version No.	Date	Author

Section 5 Declaration

Declaration (Tick all that apply)

I am a geotechnical engineer or engineering geologist as defined by the <*Regulator's geotechnical DCP*> and on behalf of the company below, I:

Yes / No	
☐ / ☐	am aware that the geotechnical report I have either prepared or am technically verifying (referenced above) is to be submitted in a support of a development application for the proposed development site (referenced above) and its findings will be relied upon by <*the Regulator*> in determining the development application.
☐ / ☐ N/A	prepared the geotechnical report referenced above in accordance with the AGS (2007c) as amended and <*Regulator's geotechnical DCP*>.
☐ / ☐ N/A	am willing to technically verify that the Geotechnical Report referenced above has been prepared in accordance with the AGS (2007c) as amended and <*Regulator's geotechnical DCP*>.
☐ / ☐ No	am willing to technically verify that the geotechnical report prepared for the development application for the site confirms the land will achieve the level of <*tolerable risk*> of slope instability as a result of the considerations described in <*add reference to specific section of*> <*Regulator's geotechnical DCP*> taking into account the total development and site disturbances proposed.
☐ / ☐ No	am willing to technically verify that the geotechnical report prepared for the site and related land being greater than two years old confirms the land will achieve the level of <*tolerable risk*> of slope instability as a result of the considerations described <*add reference to specific section of*> *of* <*Regulator's geotechnical DCP*> *taking into account the total development and site disturbances proposed.*
☐ / ☐ No	have professional indemnity insurance in accordance with <*Regulator's geotechnical DCP*> of not less than $.... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <*the Regulator*>.

Section 6 Geotechnical Engineer or Engineering Geologist Details

Company/ Organisation Name		
Name (Company Representative)	Surname:	Mr /Mrs /Other:
	Given Names:	
	Chartered Professional Status:	Registration No:
Signature		Dated: / /

Reference: AGS (2007c) *"Practice Note Guidelines for Landslide Risk Management"*. Australian Geomechanics Society, Australian Geomechanics, V42, .N1, March 2007.

Note: N/A = Not Applicable.

Page 1 of 2

FORM B

Structural/Civil/Geotechnical Engineering Declaration – <*Construction Certificate*> Application

Office Use Only		Regulator: <*Add in or change to appropriate name*>

To be submitted with the structural design forming part of an application for a <*construction certificate*>.
This form must be attached with the submission of the structural documentation required for the determination of a <*construction certificate*> or combined development application and <*construction certificate*> submission.
This form is essential, as it provides evidence to the <*PCA*> determining the <*construction certificate*>, that the structural design has been prepared or verified by a structural engineer or civil engineer as defined by <*Regulator's geotechnical DCP*> and that the structural design has been prepared in accordance with the recommendations given in the geotechnical report for the same development. This form also covers additional design documents required to cover other works not shown on the main structural/civil design drawings. This form is also essential to establish that the recommendations given in the geotechnical report have been interpreted and incorporated into the structural design as originally intended by the geotechnical engineer in preparing the geotechnical report.

Section 1 Related Application

Reference	What is the <*Regulator's*> development application number?
DA Site Address	
DA Applicant	

Section 2 Structural/Civil Design Documents

List of Structural/Civil Design Documents (More space on page two if required)	Description	Plan or Document No.	Revision or Version No.	Date	Author

Section 3 Geotechnical Report

Details	Title:	
	Author:	Dated: / /
	Author's Company/ Organisation Name:	Report Reference No:

Section 4 Declaration by Structural/Civil Engineer or Designer of Additional Design Documents in Relation to a Geotechnical Report

Declaration (Tick all that apply) Yes	No	
☐	☐	I am a structural or civil engineer as defined by the <*Regulator's geotechnical DCP*> and on behalf of the company below.
☐	☐	I have prepared the structural designs listed in Section 2 above and/or Section 6 below, in accordance with the recommendations given in the above geotechnical report.
☐	☐	I am a design engineer and have prepared Additional Design documents listed in Section 7 below in accordance with the recommendations given in the above geotechnical report.
☐	☐	I am aware that the <*PCA*> will rely on this declaration in granting a <*construction certificate*> for works to which the above structural design documents and geotechnical report relate.
☐	☐	I certify that any residential structure designed or erected in accordance with the structural design prepared by the structural engineer or civil engineer achieves the performance requirements of Clause 1.3 of the current version of AS 2870 (this must be ticked when accompanied by minimal impact certification).
☐	☐	I have professional indemnity insurance in accordance with <*Regulator's geotechnical DCP*> of not less than $.... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <*the Regulator*>.

Page 2 of 2

FORM B

Structural/Civil/Geotechnical Engineering Declaration – <*Construction Certificate*> Application

Section 5 Structural/Civil/Design Engineer Details

Company/ Organisation Name		
Name (Company Representative)	Surname:	Mr /Mrs /Other:
	Given:	
	Chartered Professional Status:	Registration No:
Signature		Dated: / /

Section 6 Ancillary Structural/Civil Design Required Prior to Completion of Geotechnical Declaration

List of Structural Design Documents Required	Description	Company Responsible	Plan or Document No.	Revision or Version No.	Date of Additional Form B *	Author
	eg. Landscaping retaining walls					
	eg. Anchor design					

Section 7 Additional Design Documents Required Prior to Completion of Geotechnical Declaration

List of Design Documents Required	Description	Company	Plan or Document No.	Revision or Version No.	Date of Additional Form B *	Author
	eg. Surface & subsoil drainage design					
	eg. Infiltration or effluent disposal					

Section 8 and 9 are not to be completed until each relevant ancillary and additional Form B has been completed and forwarded to the geotechnical engineer/engineering geologist

Section 8 Declaration in Relation to Structural/Civil Designs and Additional Design Drawings

Declaration (Tick all that apply) Yes	No	
☐	☐	I am a geotechnical engineer or engineering geologist as defined by the <*Regulator's geotechnical DCP*> and on behalf of the company below:
☐	☐	I prepared and/or technically verified the above geotechnical report and now declare that I have viewed the above listed design documents prepared for the same development.
☐	☐	I am satisfied that the recommendations given in the above geotechnical report have been incorporated into the design documents as intended.
☐	☐	I consider no additional drawings are required to show all the required works listed in the Geotechnical Report.

Section 9 Geotechnical Engineer or Engineering Geologist Details

Company/ Organisation Name		
Name (Company Representative)	Surname:	Mr /Mrs /Other:
	Given Names:	
	Chartered Professional Status:	Registration No:
Signature		Dated: / /

Note: * A separate Form B is required to be completed by the design engineer for those works listed in each of Sections 6 and 7 of this Form B.

FORM C	Page 1 of 2 Geotechnical Declaration Subdivision <*Construction Certificate*> Application	
Office Use Only		Regulator: <*Add in or change to appropriate name*>

To be submitted with an application for an engineering <*construction certificate*> for subdivision of land. This form must be attached to the application for the <*construction certificate*>.
This form is essential to verify that the geotechnical report has been prepared in accordance with <*Regulator's geotechnical DCP*> and that the author of the geotechnical report is a geotechnical engineer or engineering geologist as defined by <*Regulator's geotechnical DCP*>. Alternatively, where a geotechnical report has been prepared by a professional person not recognised by the <*Regulator's geotechnical DCP*>, then this form may be used as technical verification of the geotechnical report if signed by a geotechnical engineer or engineering geologist as defined by <*Regulator's geotechnical DCP*>.

Section 1 Related Application

Reference	What is the Regulator's Development Application Number?	
DA Site Address		
DA Applicant		

Section 2 Geotechnical Report

Details	Title:	
	Author:	Dated: / /
	Author's Company/ Organisation Name:	Report Reference No:

Section 3 Declaration

Declaration (Tick all that apply) Yes	No	
☐	☐	I am a geotechnical engineer or engineering geologist as defined by the <*Regulator's geotechnical DCP*> and on behalf of the company below:
☐	☐	I prepared the geotechnical report referenced above in accordance with the AGS (2007c) as amended and the <*Regulator's geotechnical DCP*>.
☐	☐	I am willing to technically verify that the geotechnical report referenced above has been prepared in accordance with the AGS (2007c) as amended and <*Regulator's geotechnical DCP*>.
☐	☐	I have professional indemnity insurance in accordance with <*Regulator's geotechnical DCP*> of not less than $.... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <*the Regulator*>.
☐	☐	I am aware that the geotechnical report I have either prepared or am technically verifying (referenced above) is to be submitted in support of an engineering <*construction certificate*> for subdivision of land for the proposed development site (referenced above) and its findings will be relied upon by <the *Regulator*> determining the engineering <*construction certificate*>.

FORM C	Page 2 of 2 Geotechnical Declaration Subdivision <*Construction Certificate*> Application

Section 4 Checklist

Geotechnical Requirements (Tick as appropriate, either Yes or No) Yes	No	
☐	☐	The following checklist covers the minimum requirements to be addressed in a geotechnical report in accordance with <Add reference to specific section of> <*Regulator's geotechnical DCP*>. This checklist is to accompany the report.
☐	☐	The extent and stability of proposed embankments including those acting as retarding basins <*Add reference*>
☐	☐	Recommended Geotechnical testing requirements <*Add reference*>
☐	☐	Required level of geotechnical supervision for each part of the works as defined under AS3798 – Guidelines on Earthworks for Commercial and Residential Developments <*Add reference*>
☐	☐	Compaction specification for all fill within private subdivisions <*Add reference*>
☐	☐	The level of risk to existing adjacent dwellings as a result of a construction contractor using vibratory rollers anywhere within the site the subject of these works. In the event that vibratory rollers could affect adjacent dwellings, 'high risk' areas shall be identified on a plan and the engineering plans shall be amended to indicate that no vibratory roller shall be used within that zone <*Add reference*>
☐	☐	The impact of the installation of services on overall site stability and recommendations on short term drainage methods, shoring requirements and other remedial measures that may be appropriate during installation <*Add reference*>
☐	☐	The preferred treatment of any areas of unacceptable risk within privately owned allotments <*Add reference*>
☐	☐	Requirement for subsurface drainage lines <*Add reference*>
☐	☐	Overall suitability of the engineering plans for the proposed development <*Add reference*>
☐	☐	Risk mitigation plan defined <*Add reference*>

Section 5 Geotechnical Engineer or Engineering Geologist Details

Company/ Organisation Name		
Name (Company Representative)	Surname:	Mr /Mrs /Other:
	Given Names:	
	Chartered Professional Status:	Registration No:
Signature		Dated: / /

Reference: AGS (2007c) "*Practice Note Guidelines for Landslide Risk Management*". Australian Geomechanics Society, Australian Geomechanics, V42, .N1, March 2007.

Note: <Add reference>: Add in the relevant section or page number of the listed geotechnical report which addresses each item.

FORM D	Page 1 of 2 Geotechnical Declaration Minor Impact	
Office Use Only		Regulator: *<Add in or change to appropriate name>*

This form may be used where minor construction works present minimal or no geotechnical impact on the site or related land. A geotechnical engineer or engineering geologist must inspect the site and/or review the proposed development documentation to determine if the proposed development requires a geotechnical report to be prepared to accompany the development application. Where the geotechnical engineer determines that such a report is not required then they must complete this form and attach design recommendations where required. A copy of this form with design recommendation, if required, must be submitted with the development application.

Note: In all situations, this form will need to be accompanied by Form B where the structural engineer or civil engineer certifies that any residential structure designed or erected in accordance with the plans and specifications prepared by the structural engineer or civil engineer achieve the performance requirements of Clause 1.3 of the current version of AS 2870.

Note: The use of this form does not preclude the geotechnical consultant from requiring a Geotechnical Report.

Section 1 Related Application

Reference	What is the Council Development Application Number?
DA Site Address	
DA Applicant	

Section 2 Documentation

List of Documents Reviewed *(More space on page two if required)*	Description	Plan or Document No.	Revision or Version No.	Date	Author

Section 3 Declaration

Declaration (Tick all that apply)

I am a geotechnical engineer or engineering geologist as defined by the <*Regulator's geotechnical DCP*> and I have inspected the site and reviewed the proposed development at the DA Site Address described above. As a result of my consideration of the <*Regulator's geotechnical DCP*>, of my site inspection and review of the documentation listed above, I have determined and declare that, on behalf of the company below:

Yes	No	
☐	☐	The current load-bearing capacity of the site will not be exceeded or be adversely impacted on by the proposed development, and
☐	☐	The proposed works are of such a minor nature that the requirement for geotechnical advice in the form of a geotechnical report, prepared in accordance with <*Regulator's geotechnical DCP*> is considered unnecessary for the adequate and safe design of the structural elements to be incorporated into the new works as there is no change to the current landslide risk on the site in accordance with AGS (2007c), and
☐	☐	In accordance with AS 2870 Residential Slabs and Footings, the site is to be classified as a type: _____
☐	☐	I have attached design recommendations to be incorporated in the structural design in accordance with this site classification.
☐	☐	I have professional indemnity insurance in accordance with <*Regulator's geotechnical DCP*> of not less than $.... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <*the Regulator*>.
☐	☐	I am aware that this declaration shall be used by <*The Regulator*> as an essential component in granting development consent for a structure to be erected on the site or related land without requiring submission of a geotechnical report complying with the <*Regulator's geotechnical DCP*> in support of the development application.

Reference: AGS (2007c) *"Practice Note Guidelines for Landslide Risk Management"*. Australian Geomechanics Society, Australian Geomechanics, V42, .N1, March 2007.

FORM	D	Geotechnical Declaration Minor Impact	Page 2 of 2

Section 4 Additional Documentation

List of Documents Reviewed	Description	Plan or Document No.	Revision or Version No.	Date	Author

Section 5 Geotechnical Engineer or Engineering Geologist Details

Company/ Organisation Name		
Name (Company Representative)	Surname:	Mr /Mrs /Other:
	Given Names:	
	Chartered Professional Status:	Registration No:
Signature		Dated: / /

FORM E	Page 1 of 2 Geotechnical Declaration Remediation	
Office Use Only		Regulator: <*Add in or change to appropriate name*>

This form must be submitted where development must be staged for geotechnical reasons and remediation of the site to a <*tolerable risk*> is necessary prior to any further development continuing on the site.

This form is essential, as it provides verification at each stage of the development, prior to the next stage commencing, that the remediation of the site to a <*tolerable risk*> has been carried out in accordance with the requirements of the geotechnical report and <*add reference to specific section*> of <*Regulator's geotechnical DCP*> and that no unforeseen ground conditions have been encountered which could impact on the integrity of structures on site or related land or the landslide risk. The geotechnical engineer or engineering geologist who prepared and/or verified the report must carry out site inspections as determined by the report to ensure that the design(s) documented on Form(s) B have been completed prior to signing this form.

Section 1 Related Application

Reference	What is the Development Application number?	
DA Site Address		*Development Stage (s):*
DA Applicant		

Section 2 Geotechnical Report

Details	Title:	
	Author:	Dated: / /
	Author's Company/ Organisation Name:	Report Reference No:

Section 3 Declaration

Declaration (Tick all that apply) Yes	No	
☐	☐	I am a geotechnical engineer or engineering geologist as defined by the <*Regulator's geotechnical DCP*> and, on behalf of the company below:
☐	☐	I inspected and am satisfied that the foundation materials upon which the structural elements of the development have been erected, complied with the requirements and recommendations specified in the geotechnical report for Stage (s) <*add* > of the development.
☐	☐	To the best of my knowledge, I am satisfied that Stage(s) <*add*> of the development referred to above have been carried out in accordance with all the requirements and recommendations of the above geotechnical report, and conditions of development consent relating to geotechnical issues.
☐	☐	To the best of my knowledge, I am satisfied that where changes to the development occurred during construction, those changes were carried out in accordance with all the requirements and recommendations of the above geotechnical report, conditions of development consent relating to geotechnical issues, and any site instructions or site reports issued by me as listed below.
☐	☐	I am aware that the <*PCA*> requires this certificate at the end of stage of the development specified in the development approval and prior to any further development continuing on the site and related land.
☐	☐	I am willing to technically verify that the site or related land will now achieve the level of <*tolerable risk*> of slope instability as defined by <*Regulator's geotechnical DCP*>.
☐	☐	I have professional indemnity insurance in accordance with <*Regulator's geotechnical DCP*> of not less than $.... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <*the Regulator*>.

Note: <*add*> relevant stage numbers to be inserted.

FORM	E	Geotechnical Declaration Remediation	Page 2 of 2

Section 4	List of Site Instructions and/or Site Reports Issued					
List of Documents Issued	Description/Title	Reference No.	Date	Author	Associated Design Drawings (tick as appropriate) Yes	No

Section 5	Geotechnical Engineer or Engineering Geologist Details	
Company/ Organisation Name		
Name (Company Representative)	Surname:	Mr /Mrs /Other:
	Given Names:	
	Chartered Professional Status:	Registration No:
Signature		Dated: / /

FORM F	Page 1 of 2 Geotechnical Declaration Final Structural/Civil Certificate	
Office Use Only		Regulator: <*Add in or change to appropriate name*>

This form must be submitted to the <*PCA*> at the completion of a project and prior to the issue of an <*occupation certificate*>.

This form is essential, as it provides evidence to the <*PCA*> that the development works have been carried out in accordance with the requirements of the structural design, any site inspections, and that any changes to the development occurring during construction, were carried out in accordance with all the requirements and recommendations of the structural design and geotechnical report, conditions of development consent relating to geotechnical issues, and any site instructions issued.

Section 1 Related Application

Reference	What is <*the Regulator's*> Development Application number?
DA Site Address	
DA Applicant	

Section 2 Geotechnical Report

Details	Title:	
	Author:	Dated: / /
	Author's Company/ Organisation Name:	Report Reference No:

Section 3 Structural Civil Design Documents appropriate to the 'as constructed' development

List of Structural Civil Design Documents *(More space on page two if required)*	Description	Plan or Document No.	Revision or Version No.	Date	Author

Section 4 Declaration

Declaration (Tick all that apply) Yes	No	
☐	☐	I am a structural or civil engineer as defined by the <*Regulator's geotechnical DCP*> and I prepared the above structural designs in accordance with the recommendations given in the geotechnical report described above on behalf of the company below. I:
☐	☐	inspected and am satisfied that the structural elements of the above development have been erected, and complied with the requirements and recommendations specified in the structural design and geotechnical report.
☐	☐	to the best of my knowledge, am satisfied that the above development has been carried out in accordance with all the requirements and recommendations of the structural design and above geotechnical report, and conditions of development consent relating to geotechnical issues.
☐	☐	to the best of my knowledge, am satisfied that where changes to the development occurred during construction, those changes were carried out in accordance with all the requirements and recommendations of the structural design and above geotechnical report, conditions of development consent relating to geotechnical issues, and any site instructions issued by me as listed below.
☐	☐	am aware that the <*PCA*> requires this certificate prior to issuing an<*occupation certificate*> for the above development and will rely on this certificate as verification that the above development has been erected, and complied with the requirements and recommendations specified in the structural design and geotechnical report as defined by <*Regulator's geotechnical DCP*> and in determining the <*occupation certificate*>.
☐	☐	have professional indemnity insurance in accordance with <*Regulator's geotechnical DCP*> of not less than $.... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <*the Regulator*>.

FORM F

Geotechnical Declaration
Final Structural/Civil Certificate

Section 5 List of Site Instructions Issued

List of Documents Issued	Description/Title	Reference No.	Date	Author	Associated Design Drawings	
					Yes	No

Section 6 Additional Design Documents

List of Additional Design Documents	Description	Plan or Document No.	Revision or Version No.	Date	Author

Section 7 Structural Engineer or Civil Engineer Details

Company/ Organisation Name		
Name (Company Representative)	Surname:	Mr /Mrs /Other:
	Given Names:	
	Chartered Professional Status:	Registration No:
Signature		Dated: / /

FORM G

Page 1 of 2

Geotechnical Declaration
Final Geotechnical Certificate

Office Use Only		Regulator: *<Add in or change to appropriate name>*

This form must be submitted to the <*PCA*> at the completion of a project and prior to the issue of an <*occupation or subdivision certificate*>.
This form is essential, as it provides verification that the development works have been carried out in accordance with the requirements of the geotechnical report during construction, and any site inspections, and that no unforeseen ground conditions have been encountered which could have an impact on the integrity of structures on site or related land and any subsequent geotechnical requirements introduced during the construction process.

Section 1 Related Application

Reference	What is the Development Application number?
DA Site Address	
DA Applicant	

Section 2 Geotechnical Report

Details	Title:	
	Author:	Dated: / /
	Author's Company/ Organisation Name:	Report Reference No:

Section 3 Work as Executed Drawings & Ongoing Maintenance Plans relevant to Geotechnical Risk Management

List of Documents (more space on page 2 if required)

Description	Plan or Document No.	Revision or Version No.	Date	Author

Section 4 Declaration

Declaration (Tick all that apply)

Yes	No	
☐	☐	I am a geotechnical engineer or engineering geologist as defined by the <*Regulator's geotechnical DCP*> and I prepared or verified the geotechnical report as described above on behalf of the company below. I:
☐	☐	inspected and am satisfied that the foundation materials upon which the structural elements of the development have been erected, complied with the requirements and recommendations specified in the geotechnical report.
☐	☐	to the best of my knowledge, am satisfied that the development referred to above has been carried out in accordance with all the requirements and recommendations of the above geotechnical report, and conditions of development consent relating to geotechnical issues.
☐	☐	to the best of my knowledge, am satisfied that where changes to the development occurred during construction, those changes were carried out in accordance with all the requirements and recommendations of the above geotechnical report, conditions of development consent relating to geotechnical issues, and any site instructions or site reports issued by me as listed below.
☐	☐	am aware that the <*PCA*> requires this certificate prior to issuing an occupation or subdivision certificate for the above development and will rely on this certificate as verification that the above development has achieved the necessary level of <*tolerable risk*> as defined by <*Regulator's geotechnical DCP*> and in determining the <*occupation or subdivision certificate*>.

FORM G

Geotechnical Declaration
Final Geotechnical Certificate

Section 5 List of Site Reports or Site Instructions Issued

List of Documents Issued	Description/Title	Reference No.	Date	Author	Associated Design Drawings	
					Yes	No

Section 6 Additional Work as Executed Drawings and Ongoing Maintenance Plans relevant to Geotechnical Risk Management

List of Additional Documents	Description	Plan or Document No.	Revision or Version No.	Date	Author

Section 7 Geotechnical Engineer or Engineering Geologist Details

Company/ Organisation Name		
Name (Company Representative)	Surname:	Mr /Mrs /Other:
	Given Names:	
	Chartered Professional Status:	Registration No:
Signature		Dated: / /

FORM H	Page 1 of 2 Geotechnical Declaration <*Building Certificate*> or Order	
Office Use Only		Regulator: <*Add in or change to appropriate name*>
This form is to be submitted with Application for a <*Building Certificate*> or in response to an order.		
Section 1	Related Application	
Reference	What is the *Regulator's DA / BA / Order* number?	
Site Address		
Applicant		
Section 2	Geotechnical Report	
Details	Title:	
	Author:	Dated: / /
	Author's Company/ Organisation Name:	Report Reference No:
Section 3	Declaration	
Declaration (Tick all that apply) Yes ☐ No ☐	I am a geotechnical engineer or engineering geologist as defined by the <*Regulator's geotechnical DCP*> and I prepared or verified the geotechnical report as described above on behalf of the company below. I:	
☐ ☐	have inspected the site and existing development and am satisfied that both the site and development achieves <*tolerable risk*> level requirement of the <*Regulator's geotechnical DCP*>. The attached report provides details of the assessment in accordance with the <*Regulator's geotechnical DCP*>. The report also contains recommendations as to any reasonable and practical measures that can be undertaken to reduce foreseeable risk.	
☐ ☐	have inspected the site of the existing development. The attached report details the remedial actions required to be undertaken prior to me being prepared to certify that the site and the development achieves the <*tolerable risk*> criteria required by the <*Regulator's geotechnical DCP*>.	
☐ ☐	to the best of my knowledge, am satisfied that where changes to the development occurred during construction, those changes were carried out in accordance with all the requirements and recommendations of the above geotechnical report, conditions of development consent relating to geotechnical issues, and any site reports or site instructions issued by me as listed below.	
☐ ☐	am aware that the <*PCA*> requires this certificate prior to issuing a <*Building Certificate*> for the above development and will rely on this certificate as verification that the development has achieved the necessary level of <*tolerable risk*> as defined by <*Regulator's geotechnical DCP*> and in determining the <*occupation or subdivision certificate*>.	
☐ ☐	have professional indemnity insurance in accordance with <*Regulator's geotechnical DCP*> of not less than $.... million, being in force for the year in which the report is dated, with retroactive cover under this insurance policy extending back to the engineer's first submission to <*the Regulator*>.	

FORM H

Page 2 of 2

Geotechnical Declaration
<*Building Certificate*> or Order

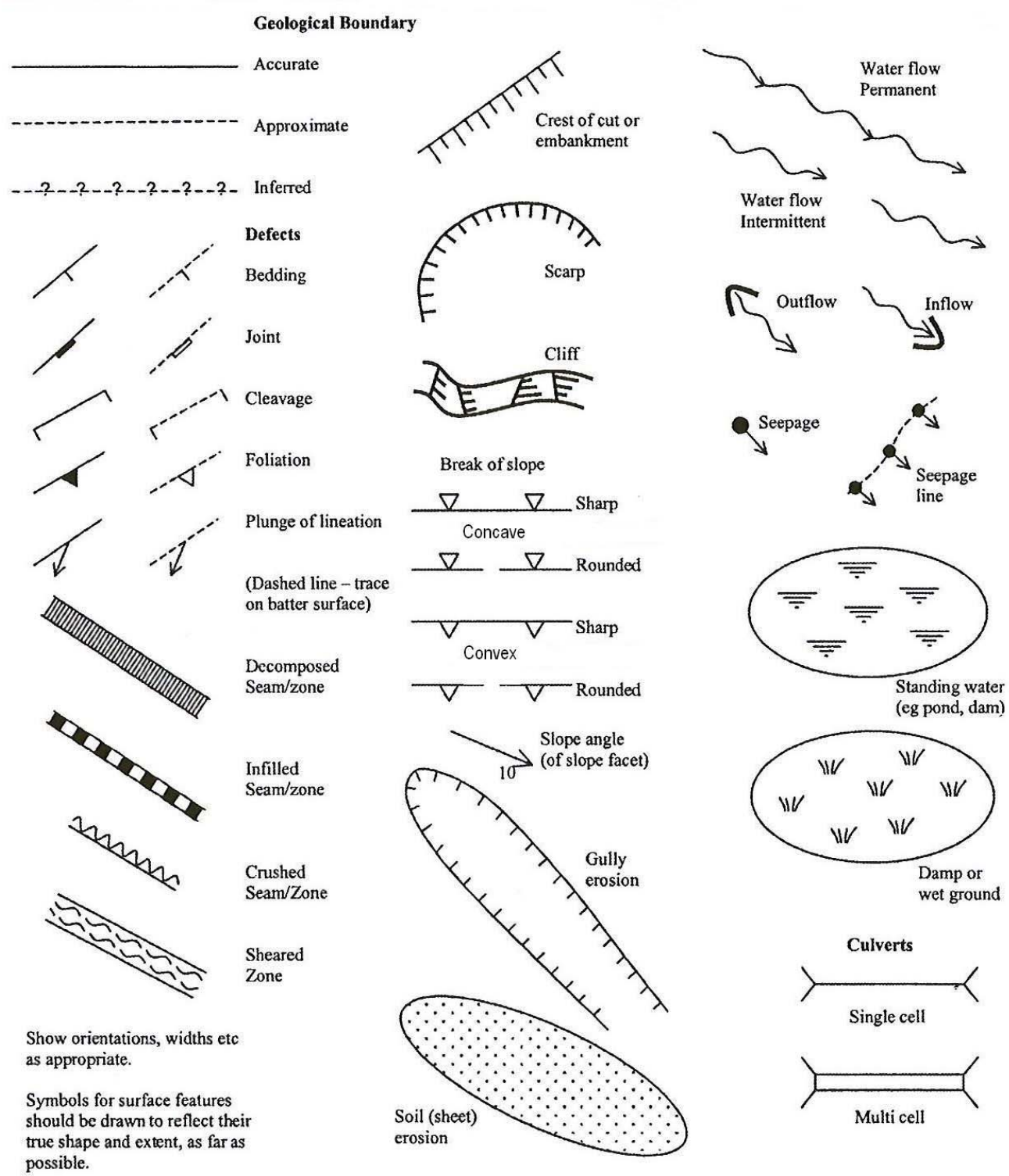
Section 4 List of Site Reports or Site Instructions Issued

List of Documents Issued	Description/Title	Reference No.	Date	Author	Associated Design Drawings	
					Yes	No

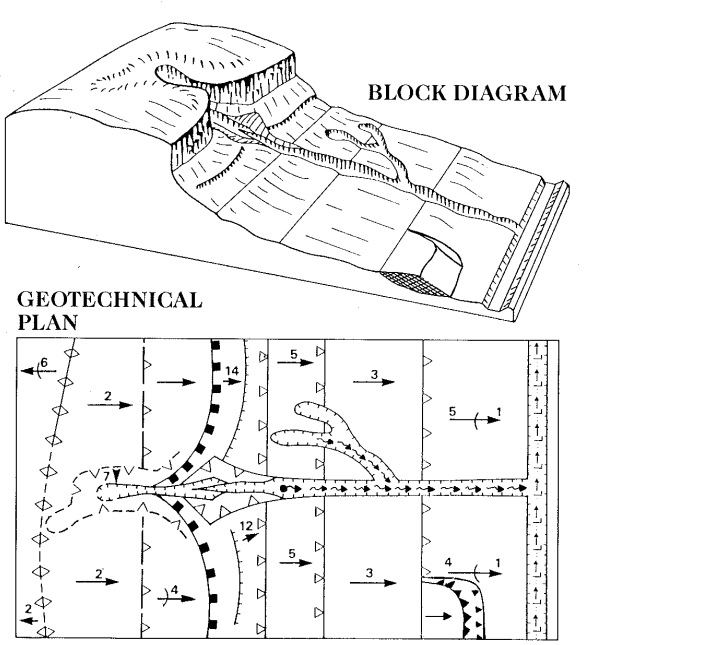
Section 5 Geotechnical Engineer or Engineering Geologist Details

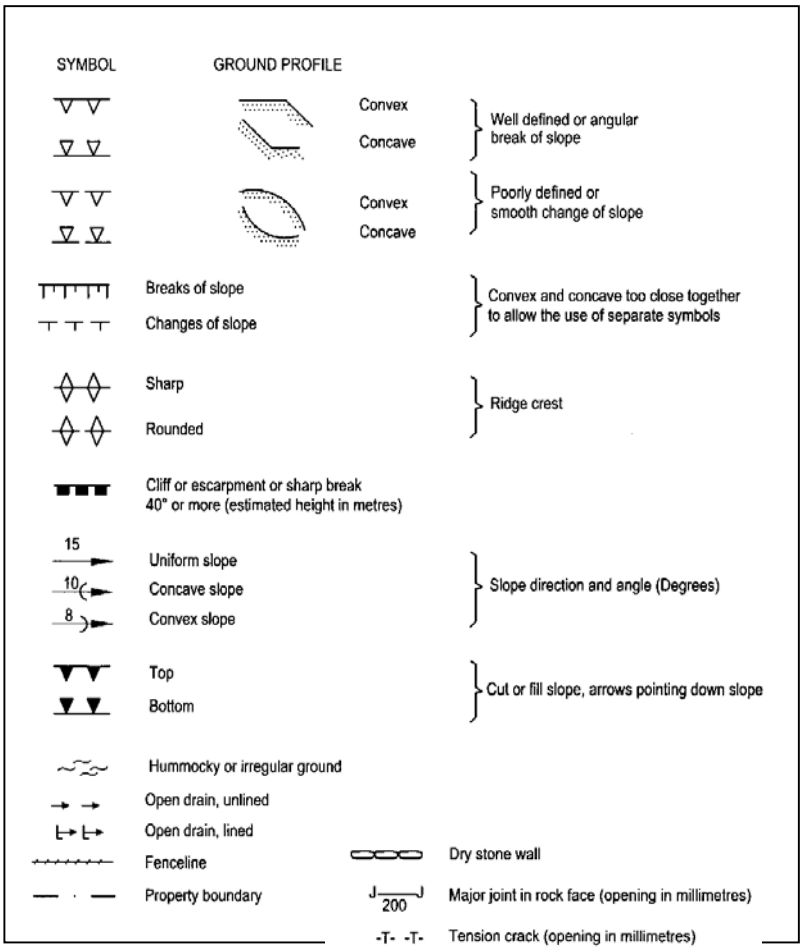
Company/ Organisation Name		
Name (Company Representative)	Surname:	Mr /Mrs /Other:
	Given Names:	
	Chartered Professional Status:	Registration No:
Signature		Dated: / /

APPENDIX E - GEOLOGICAL AND GEOMORPHOLOGICAL MAPPING SYMBOLS AND TERMINOLOGY



Examples of Mapping Symbols (after Guide to Slope Risk Analysis Version 3.1 November 2001, Roads and Traffic Authority of New South Wales).





Example of Mapping Symbols
(after V Gardiner & R V Dackombe (1983).Geomorphological Field Manual. George Allen & Unwin).

APPENDIX F- EXAMPLE OF VULNERABILITY VALUES

SUMMARY OF HONG KONG VULNERABILITY RANGES FOR PERSONS, AND RECOMMENDED VALUES FOR LOSS OF LIFE FOR LANDSLIDING IN SIMILAR SITUATIONS

The following table is adapted from P J Finlay, G R Mostyn & R Fell (1999). *Landslides: Prediction of Travel Distance and Guidelines for Vulnerability of Persons*. Proc 8th. Australia New Zealand Conference on Geomechanics, Hobart. Australian Geomechanics Society, ISBN 1 86445 0029, Vol 1, pp.105-113.

Case	Range in Data	Recommended Value	Comments
Person in Open Space			
If struck by a rockfall	0.1 – 0.7	0.5	May be injured but unlikely to cause death
If buried by debris	0.8 – 1.0	1.0	Death by asphyxia almost certain
If not buried	0.1 – 0.5	0.1	High chance of survival
Persons in a Vehicle			
If the vehicle is buried/crushed	0.9 – 1.0	1.0	Death is almost certain
If the vehicle is damaged only	0 – 0.3	0.3	High chance of survival
Person in a Building			
If the building collapses	0.9 – 1.0	1.0	Death is almost certain
If the building is inundated with debris and the person buried	0.8 – 1.0	1.0	Death is highly likely
If the debris strikes the building only	0 – 0.1	0.05	Very high chance of survival

EXAMPLE OF VULNERABILITY VALUES FOR DESTRUCTION OF PEOPLE, BUILDINGS AND ROADS

The following table is adapted from Marion Michael-Leiba, Fred Baynes, Greg Scott & Ken Granger (2002). *Quantitative Landslide Risk Assessment of Cairns*. Australian Geomechanics, June 2002.

Geomorphic Unit	Vulnerability Values		
	People	Buildings	Roads
Hill slopes	0.05	0.25	0.3
Proximal debris fan	0.5	1.0	1.0
Distal debris fan	0.05	0.1	0.3

EXAMPLE OF VULNERABILITY VALUES FOR LIFE FOR ROCKFALLS AND DEBRIS FLOWS FOR LAWRENCE HARGRAVE DRIVE PROJECT, COALCLIFF TO CLIFTON AREA, AUSTRALIA

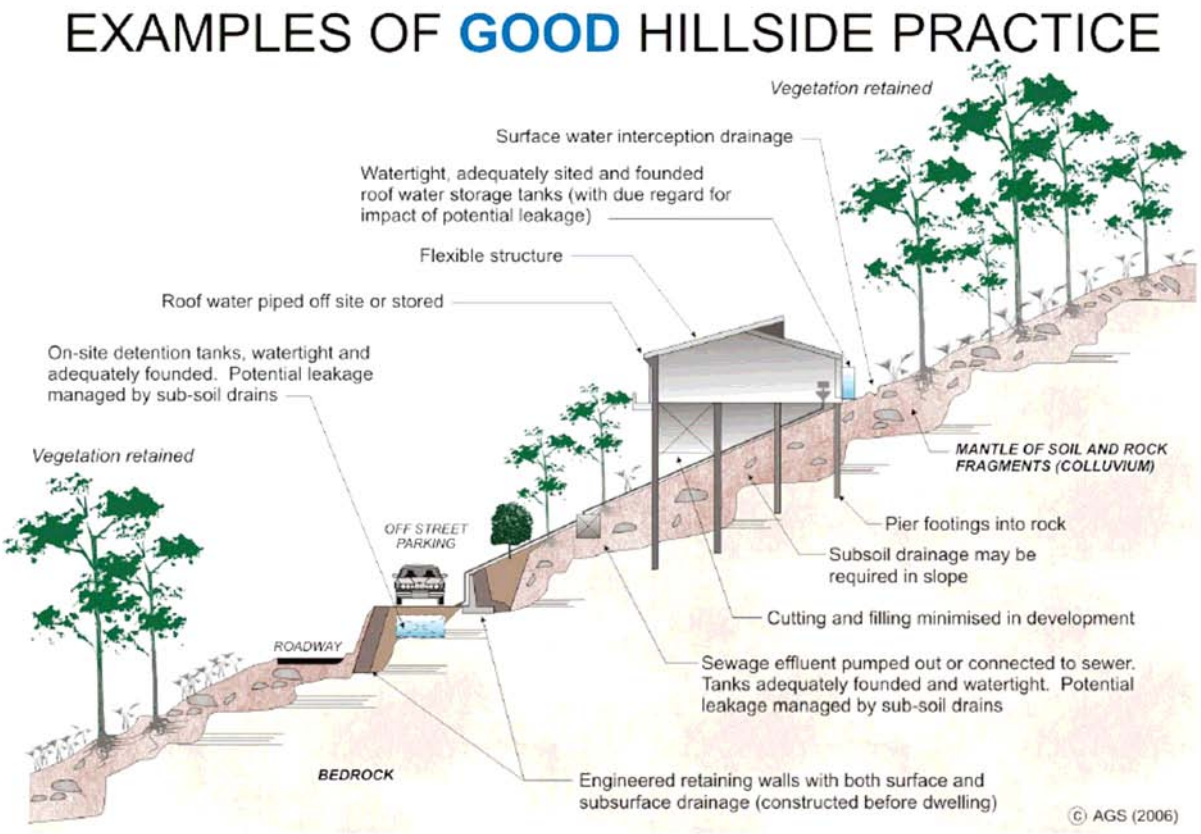
The following table is adapted from R A Wilson, A T Moon, M Hendricks & I E Stewart (2005). *Application of quantitative risk assessment to the Lawrence Hargrave Drive Project, New South Wales,Australia*. Landslide Risk Management - Hungr, Fell, Couture & Eberhardt (eds) 2005. Taylor & Francis Group, London, ISBN 04 1538 043X.

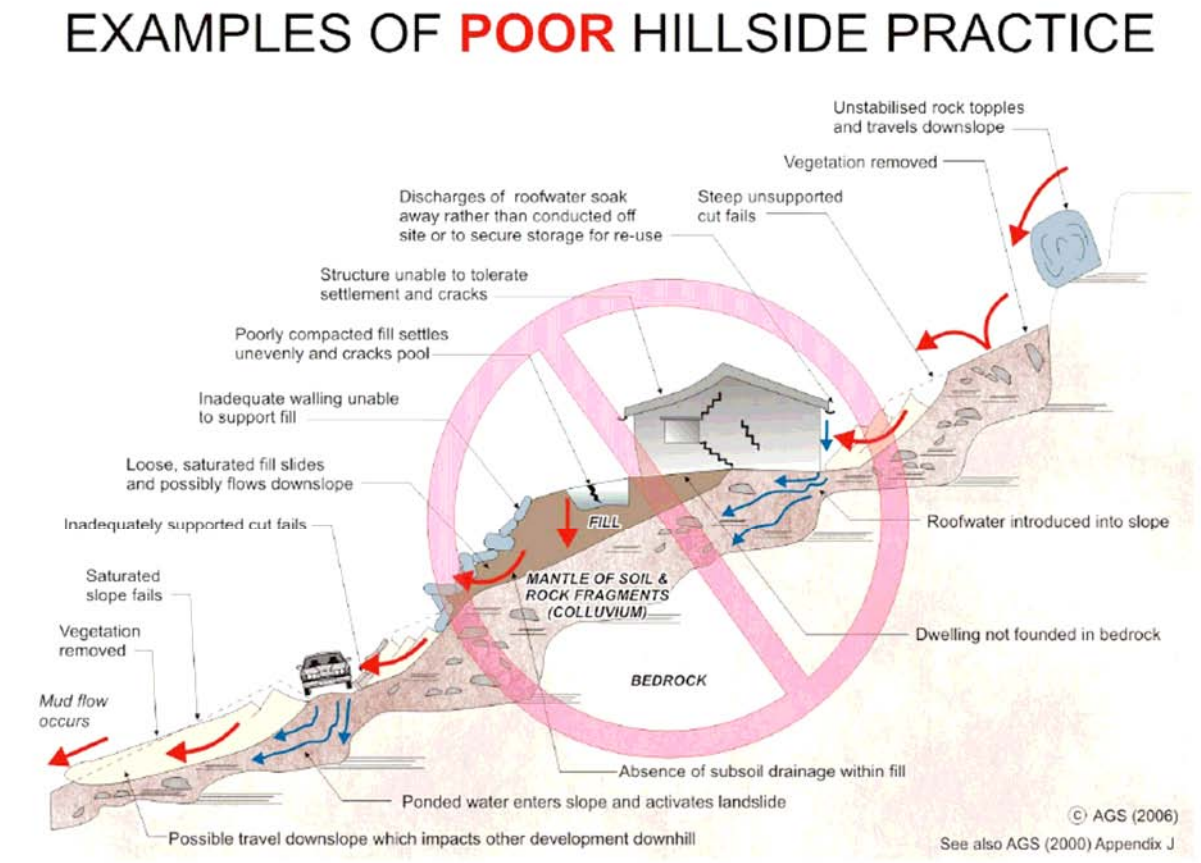
Order of magnitude of landslide crossing road (m^3)	**Rockfalls from Scarborough Cliff**		**Debris flow from Northern Amphitheatre**	
	Landslide hits car	**Car hits landslide**	**Landslide hits car**	**Car hits landslide**
0.03	0.05	0.006	–	–
0.3	0.1	0.002	–	–
3	0.3	0.03	0.001	–
30	0.7	0.03	0.01	0.001
300	1	0.03	0.1	0.003
3,000	1	0.03	1	0.003

NOTE: The above data should be applied with common sense, taking into account the circumstances of the landslide being studied. Judgment may indicate values other than the recommended value are appropriate for a particular case.

APPENDIX G - SOME GUIDELINES FOR HILLSIDE CONSTRUCTION

	GOOD ENGINEERING PRACTICE	*POOR ENGINEERING PRACTICE*
ADVICE		
GEOTECHNICAL ASSESSMENT	Obtain advice from a qualified, experienced geotechnical practitioner at early stage of planning and before site works.	Prepare detailed plan and start site works before geotechnical advice.
PLANNING		
SITE PLANNING	Having obtained geotechnical advice, plan the development with the risk arising from the identified hazards and consequences in mind.	Plan development without regard for the Risk.
DESIGN AND CONSTRUCTION		
HOUSE DESIGN	Use flexible structures which incorporate properly designed brickwork, timber or steel frames, timber or panel cladding. Consider use of split levels. Use decks for recreational areas where appropriate.	Floor plans which require extensive cutting and filling. Movement intolerant structures.
SITE CLEARING	Retain natural vegetation wherever practicable.	Indiscriminately clear the site.
ACCESS & DRIVEWAYS	Satisfy requirements below for cuts, fills, retaining walls and drainage. Council specifications for grades may need to be modified. Driveways and parking areas may need to be fully supported on piers.	Excavate and fill for site access before geotechnical advice.
EARTHWORKS	Retain natural contours wherever possible.	Indiscriminatory bulk earthworks.
CUTS	Minimise depth. Support with engineered retaining walls or batter to appropriate slope. Provide drainage measures and erosion control.	Large scale cuts and benching. Unsupported cuts. Ignore drainage requirements
FILLS	Minimise height. Strip vegetation and topsoil and key into natural slopes prior to filling. Use clean fill materials and compact to engineering standards. Batter to appropriate slope or support with engineered retaining wall. Provide surface drainage and appropriate subsurface drainage.	Loose or poorly compacted fill, which if it fails, may flow a considerable distance including onto property below. Block natural drainage lines. Fill over existing vegetation and topsoil. Include stumps, trees, vegetation, topsoil, boulders, building rubble etc in fill.
ROCK OUTCROPS & BOULDERS	Remove or stabilise boulders which may have unacceptable risk. Support rock faces where necessary.	Disturb or undercut detached blocks or boulders.
RETAINING WALLS	Engineer design to resist applied soil and water forces. Found on rock where practicable. Provide subsurface drainage within wall backfill and surface drainage on slope above. Construct wall as soon as possible after cut/fill operation.	Construct a structurally inadequate wall such as sandstone flagging, brick or unreinforced blockwork. Lack of subsurface drains and weepholes.
FOOTINGS	Found within rock where practicable. Use rows of piers or strip footings oriented up and down slope. Design for lateral creep pressures if necessary. Backfill footing excavations to exclude ingress of surface water.	Found on topsoil, loose fill, detached boulders or undercut cliffs.
SWIMMING POOLS	Engineer designed. Support on piers to rock where practicable. Provide with under-drainage and gravity drain outlet where practicable. Design for high soil pressures which may develop on uphill side whilst there may be little or no lateral support on downhill side.	
DRAINAGE		
SURFACE	Provide at tops of cut and fill slopes. Discharge to street drainage or natural water courses. Provide general falls to prevent blockage by siltation and incorporate silt traps. Line to minimise infiltration and make flexible where possible. Special structures to dissipate energy at changes of slope and/or direction.	Discharge at top of fills and cuts. Allow water to pond on bench areas.
SUBSURFACE	Provide filter around subsurface drain. Provide drain behind retaining walls. Use flexible pipelines with access for maintenance. Prevent inflow of surface water.	Discharge roof runoff into absorption trenches.
SEPTIC & SULLAGE	Usually requires pump-out or mains sewer systems; absorption trenches may be possible in some areas if risk is acceptable. Storage tanks should be water-tight and adequately founded.	Discharge sullage directly onto and into slopes. Use absorption trenches without consideration of landslide risk.
EROSION CONTROL & LANDSCAPING	Control erosion as this may lead to instability. Revegetate cleared area.	Failure to observe earthworks and drainage recommendations when landscaping.
DRAWINGS AND SITE VISITS DURING CONSTRUCTION		
DRAWINGS	Building Application drawings should be viewed by geotechnical consultant	
SITE VISITS	Site Visits by consultant may be appropriate during construction/	
INSPECTION AND MAINTENANCE BY OWNER		
OWNER'S RESPONSIBILITY	Clean drainage systems; repair broken joints in drains and leaks in supply pipes. Where structural distress is evident see advice. If seepage observed, determine causes or seek advice on consequences.	


EXAMPLES OF GOOD HILLSIDE PRACTICE
Vegetation retained
Surface water interception drainage
Watertight, adequately sited and founded roof water storage tanks (with due regard for impact of potential leakage)
Flexible structure
Roof water piped off site or stored
On-site detention tanks, watertight and adequately founded. Potential leakage managed by sub-soil drains
Vegetation retained
MANTLE OF SOIL AND ROCK FRAGMENTS (COLLUVIUM)
OFF STREET PARKING
Pier footings into rock
Subsoil drainage may be required in slope
Cutting and filling minimised in development
ROADWAY
Sewage effluent pumped out or connected to sewer. Tanks adequately founded and watertight. Potential leakage managed by sub-soil drains
BEDROCK
Engineered retaining walls with both surface and subsurface drainage (constructed before dwelling)
© AGS (2006)


EXAMPLES OF POOR HILLSIDE PRACTICE
Unstabilised rock topples and travels downslope
Vegetation removed
Discharges of roofwater soak away rather than conducted off site or to secure storage for re-use
Steep unsupported cut fails
Structure unable to tolerate settlement and cracks
Poorly compacted fill settles unevenly and cracks pool
Inadequate walling unable to support fill
Loose, saturated fill slides and possibly flows downslope
Inadequately supported cut fails
FILL
Roofwater introduced into slope
Saturated slope fails
MANTLE OF SOIL & ROCK FRAGMENTS (COLLUVIUM)
Vegetation removed
Dwelling not founded in bedrock
BEDROCK
Mud flow occurs
Absence of subsoil drainage within fill
Ponded water enters slope and activates landslide
© AGS (2006)
Possible travel downslope which impacts other development downhill
See also AGS (2000) Appendix J

WHANGAREI DISTRICT COUNCIL

4.2 Parking Restrictions – Tania Place

Meeting: Infrastructure Committee
Date of meeting: 10 May 2018
Reporting officer: Nick Marshall (Senior Roading Engineer – Traffic & Parking)

1 Purpose

To seek Council's approval for implementing 'No Stopping at All Times' parking restrictions in Tania Place.

2 Recommendation

That the Infrastructure Committee approves the pursuant to the Whangarei District Council Parking and Traffic Bylaw 2017 Clause 14, the parking of vehicles be prohibited at all times as per the attached plan and the RAMM schedule below:

Road Name: Tania Place		
Road ID: 404		
	RAMM Displacement	
Side	Start	End
RHS	107	118

3 Background

Council has received a request from the Tikipunga Primary School and the Whangarei Police to replace the existing 'P1' (1-minute maximum parking time) restrictions in the end of Tania Place by 'No Stopping at All Times' (NSAAT) restrictions.

Tania Place is a local residential cul-de-sac street in Tikipunga which currently has NSAAT restrictions on the western side for entire length and partially around the turning head. In 2011, the school and the police requested council to provide a designated parking area on Tania Place for parents to drop off and pick up their children to and from the school, in response to which the Council provided two P1 parking spaces on Tania Place by removing NSAAT restrictions at that location.

4 Discussion

4.1 Issues

The school and the police have now returned to Council requesting rescindment of these P1 parking restrictions and restoration of the previous NSAAT restrictions because the changes they previously requested have proven unfavourable to the school and the police. People park in the P1 parking zone for more than one minute especially in the afternoons when parents wait outside the school to pick up their children. As reported by the police, the P1 area is regularly misused by parking offenders and vehicles are unable to turn around at the turning head because of the vehicles parked in that area. Furthermore, because of this unique situation, the infringement of offending vehicles is not justified by the police which also makes it difficult for them to enforce the adjoining NSAAT parking restrictions.

4.2 Proposal

It is proposed that the current P1 parking restrictions in Tania Place be replaced by NSAAT restrictions i.e. rescind the parking changes made in 2011 and restore the pre-existing parking restrictions

4.3 Consultation

The proposed parking changes were requested by the Tikipunga Primary School and the Whangarei Police. Moreover, Tikipunga Kindergarten was also recognised as a directly affected party and when consulted supported the proposed changes.

4.4 Impacts

The proposal will effectively remove two P1 parking spaces in Tania Place leaving behind a total of 8 unrestricted parking spaces in the street. However, there are more than 100 unrestricted parking spaces available within 5 minutes walking distance from Tania Place.

4.5 Photos

Photo 1: Turning head, turning bus and existing parking restrictions

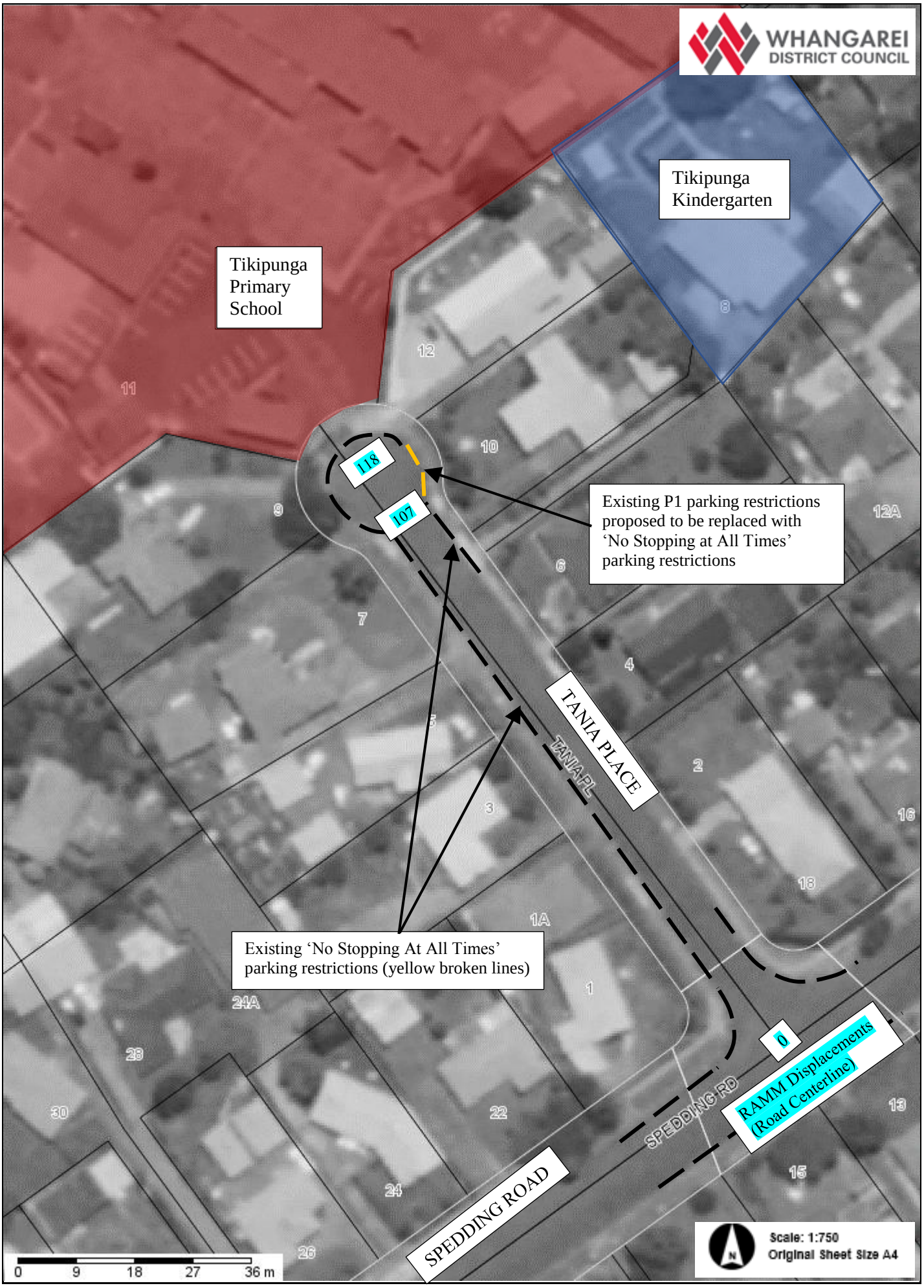
Photo 2: Existing P1 parking restrictions proposed to be removed

Photo 3: Pre-existing parking restrictions proposed to be restored

5 Attachment

1. Proposed Parking Restrictions on Tania Place

ATTACHMENT 1 – PROPOSED PARKING RESTRICTIONS ON TANIA PLACE



WHANGAREI DISTRICT COUNCIL

4.3 Western Hills Drive – Russell road to Rust Ave vegetation control

Meeting: Infrastructure Committee

Date of meeting: 10 May 2018

Reporting officer: Sue Hodge (Manager – Parks & Recreation)

1 Purpose

For the Infrastructure Committee to approve un-budgeted expenditure for vegetation control of part of Western Hills Drive, Whangarei

2 Recommendation

That the Infrastructure Committee approves $62,000 plus GST, of un-budgeted expenditure for vegetation control on Western Hills Drive from Russell Road – Rust Ave.

3 Background

The western banks of the Western Hills Drive, between Rust Avenue and Russell Road, has become very weed infested, particularly with cherry trees and gorse. This area borders onto the Coronation Reserve and the steep embankments and steep nature of the reserve make the control of vegetation above Western Hills Drive very difficult.

In the past, Council have mown the grass verges however there has been no programmed maintenance of the hillside.

In the last year or so with all the roadworks along this stretch of road the site has been inaccessible to our maintenance contractor. The changes have also made access to the site unsafe and the machinery used in the past now unsuitable.

4 Discussion

In the draft 2018-2028 Long Term Plan (LTP) there is budget to control vegetation and improve the entrance ways to the city. Year 1 is to complete one off clearance and Year 2 is for on-going maintenance.

Over the last few years much of the Western Hills Drive has been up-graded and finished to a high standard. Western Hills Drive is a high-profile route through the city and northwards. The additional vegetation control budget does not include maintenance of this area.

The western side of Western Hills Drive, between Rust Avenue and Russel Road, has become very weed infested, particularly with cherry trees and gorse. This area borders onto the Coronation Reserve and the steep embankments and steep nature of the reserve make the control of vegetation above Western Hills Drive very difficult.

In the last year or so with all the roadworks along this stretch of road the site has been inaccessible to our maintenance contractor. The changes have also made access to the site unsafe and the machinery used in the past now unsuitable.

We have a methodology from a contractor that will tidy up the area and make ongoing maintenance possible. This will include removing all large unwanted trees that are mostly large pest trees, but also a few planted trees that are now too close to the road and will pose maintenance issues going forward. This will be followed up with a side arm flail mower that can reach 5 – 6m up the bank. The pest plants above this reach will be controlled manually to the native bush line.

The area at the north end of this site is now behind a safety barrier and can no longer be mown. It is proposed that this area will be planted this winter with low maintenance shrubs. Once the site is under control, the grass verge will be cut monthly using a side arm machine driving along the road. The banks will be cut twice a year using a side arm flail mower. The areas above the reach of the mower will be monitored and tidied up as and when required.

The planted area behind the safety barrier will be maintained by Recreational Services as part of their Maintenance Contract.

4.1 Financial/budget considerations

The one-off clean-up will cost $62,000 plus GST including all traffic management costs. It is recommended this is funded from 2017/2018 surplus Infrastructure Group operational budget.

The on-going costs will be $6,000 plus GST per annum and this can be funded from the Parks and Garden operational budgets.

5 Significance and engagement

The decisions or matters of this Agenda do not trigger the significance criteria outlined in Council's Significance and Engagement Policy, and the public will be informed via publication on the website.

WHANGAREI DISTRICT COUNCIL

5.1 Contracts Approved Under Delegated Authority

Meeting: Infrastructure Committee
Date of meeting: 10 May 2018
Reporting officer: Simon Weston (General Manager Infrastructure)

1 Purpose

For the Infrastructure Committee to note Infrastructure contracts awarded under Chief Executive and General Manager delegated authority

2 Recommendation

That the Infrastructure Committee note the Infrastructure contracts awarded under Chief Executive and General Manager delegated authority.

3 Background

Table 1 (below) records Infrastructure contracts awarded under Chief Executive and General Manager delegated authority. Attachment 1 provides a summary of the award process for each contract, and a brief description of the works being undertaken

Table 1: Infrastructure Contracts Awarded Under Delegated Authority

1. Water	
CON17045	Watermain Replacements 17/18
CON17077	Ruddells Raw Watermain Pipe Bridges Rehabilitation

4 Significance and engagement

The decisions or matters of this Agenda do not trigger the significance criteria outlined in Council's Significance and Engagement Policy, and the public will be informed via Agenda publication on the website.

5 Attachments

1. Summary of Contracts Approved Under Delegated Authority May 2018.

1.0 Summary of Contracts Approved Under Delegated Authority

This attachment provides a summary of the award process, and works being undertaken, for Infrastructure contracts awarded under Chief Executive and General Manager delegated authority.

1.0 Water

CON17045
Watermain Replacements 17/18

Background

Water Services have identified three sections of watermain and rider main that are in poor condition and in need of replacement. The mains are in Kent Road (Kensington), Tikorangi Place (Morningside) and Albany Road (One Tree Point). These mains have experienced multiple failures and in the case of Kent Road there are very low flows from fire hydrants due to tuberculations in the cast iron.

Contract Tender

The contract was publicly advertised and closed on 22 March 2018. Two tenders were received as follows:

Tenderer	Amount
Forté Civil	$335,242.40
The Watertight Company	$375,943.42
Engineers Estimate	**$320,641.74**

The tender evaluation method was lowest price conforming. The Forté Civil tender was found to be conforming.

In the past year, Forté have been successful in qualifying as a registered contractor for Water Services and have nominated Watco Plumbing as their Licensed Contractor to undertake the connections to the live reticulation.

Financial

The lowest tender is 5% higher than the Engineers Estimate.

Within this year's budget there is approximately $500,000 left, which will cover the cost of this work.

Chief Executive Approval

Contract 17045, for Watermain Replacements 17/18, was awarded to Forté Civil for the tendered sum of three hundred and thirty five thousand, two hundred and forty two dollars and forty cents ($335,242.40) excluding GST.

CON17077
Ruddells Raw Watermain Pipe Bridges Rehabilitation

Background

Contract 17077, Ruddells Raw Watermain Pipe Bridges Rehabilitation, was awarded on 15 December 2017.The contract's purpose is to rehabilitate the Queen and King Post bridge structures by replacing severely corroded critical connection members, removing built-up ground cover to exposed structural elements and removing vegetation around the structures. The structure and pipe bridges are then to be fully coated with protective covering.

Reason for Variation Request

A summary of the variations and cost increases during the course of the contract is also included below:

1. Two exposed pipe joints close to, but not within the extent of the pipe bridge rehab works, were discovered to be badly corroded with water seeping out (photo below). The Engineer considered that it would be prudent and cost effective to attend to these defects while the bridge works were being undertaken.

2. Once the debris and vegetation had been cleared around the pipe bridges, the structural engineer noticed that an additional King Post Tension Rod and Plate was needed to make the structure sound. This was already priced in the scope of works and the contractor was instructed to proceed.

Description	Amount
VO 001 - 2 x pipe repairs	$10,467.37
VO 002 – Supply and Install additional King Post Tension Rod & Plate	$3,450.00

The above variations resulted in an increase to the cost of works of $823.41, and an overall contract value increase from $109,786.89 to $110,610.30.

Financial

Within this year's budget there is $1,051,650. The current commitment is $460,000 for CON17066, Ruddells Raw Watermain replacement. This leaves approximately $592,000 in PJ 00156 CX Ruddells Raw Watermain Renewal, which will cover the cost of this work.

General Manager Approval

The General Manager Infrastructure authorised a variation to Contract 17077, Ruddells Raw Watermain Pipe Bridges Rehabilitation, to increase the contract value to one hundred and ten thoudand, six hunred and ten dollars and thirty cents ($110,610.30) excluding GST.

WHANGAREI DISTRICT COUNCIL

5.2 Infrastructure Capital Projects Carry Forwards Report for the month ending 31 March 2018

Meeting: Infrastructure Committee

Date of meeting: 10 May 2018

Reporting officer: Simon Weston (General Manager Infrastructure)

1 Purpose

To provide the Infrastructure Capital Projects Report and Carry Forwards Report for the month ending 31 March 2018.

2 Recommendation

That the Infrastructure Committee notes the Infrastructure Capital Projects Report and Carry Forwards Report for the month ending 31 March 2018.

3 Background

This report provides an update on Infrastructure capital projects expenditure for the year to date compared to budget, as well as the forecast spend for the year and carry forwards against budget. The carry forwards report further details the specific projects likely to have carry forwards.

In the previous Carry Forwards Report, forecast carry forwards for infrastructure were estimated between $8.6m and 11.8m, and are now forecast to be $12.3m. The increase being mainly due to the supply of LED lighting from Italy, cycleway construction in the rail corridor and a delay to the urban intersection upgrades.

4 Discussion

The Infrastructure capital projects expenditure as at 31 March 2018 is $28.3m out of the $39.7m year to date budget. Infrastructure is forecasting to spend a total of $47.1m at year end against the $58.3m budget, with a forecast carry forward of $12.3m to the next financial year. However, if we continue to get bad weather or our negotiations with other parties are slower than expected, then carry forwards could reach $14.0m. This is our “downside Scenario”.

Major projects completed to-date in the 2017/18 financial year include:

- Mander Park Entranceway
- H&H Slipway Pocket Park
- Hatea Loop Lighting & CCTV
- Parihaka Mountain Bike Tracks
- Parihaka Track Upgrades (Drummond Track)

- Dog Park Drainage Upgrade
- Hikurangi Hardcourts
- Waipu Pontoon
- Sandy Bay Carpark Upgrade & Erosion Control
- Pataua North Boat Ramp
- Ngunguru Seawall Stage 1 (Te Maika)
- Water Reservoir Rehabilitations 17/18
- Botanica Building Upgrades
- Tarewa Rd – Jubilee Park Sewer Diversion

Major projects currently under construction include:

- LED Streetlight Upgrades
- Kamo Cycleway
- Tarewa Park Wastewater Storage Tank
- Ruddells Raw Water Line Renewal
- Otaika Sports Field Renewals
- Port Rd - Balance Site Demolition
- Town Basin Amenity Dredging
- Parihaka Track Renewals (Dobbie and Hokianga Tracks)

The main variances against budget include:

- Cycleways is currently underspent by $0.9m due to delays with NZ Rail and is forecast to carry forward $0.8m to the next financial year.

- Land for Roads is overspent by $0.8m due to the unbudgeted prior commitment to purchase land for roading in new growth areas.

- Sealed Road Pavement Rehabilitation is currently underspent by $2.0m and forecast to be underspent by $3.2m at year end due to the NZTA approved programme being less than budgeted and funding transferred to resealing and other programmes (see below).

- Sealed Road Resurfacing is currently $0.9m overspent and forecast to be overspent at year end by $0.5m due to reallocation of funding from seal rehabilitations as per the approved NZTA programme (mentioned above).

- Minor Improvements to Network is currently underspent by $0.5m forecast to be overspent at year end by $0.6m, as per NZTA approved programmes mentioned above.

- Urban Intersections Upgrades are currently underspent by $0.6m to-date and forecast to carry forward $0.8m at year end due to receiving no suitable tenders for the Porowini Intersection Upgrades. This will be retendered in 2018/19.

- LED Streetlight Upgrades is forecast to be underspent and carry forward $2.7m at year end due to delay in the supply of fittings from Italy.

- Wastewater City Service Level Improvements is forecast to be underspent and carry forward $1.1m at year end. This is primarily due to adverse weather extending construction timeframe on Tarewa storage tank beyond June, and aligning Tarewa Trunk Sewer project to NZTA project timeframes.

- Neighbourhood & Public Garden Renewals are currently underspent by $0.5m to date and forecast to be underspent by $0.4m due with carry forwards of $0.3m for the Laurie Hall Park which has been delayed to avoid ANZAC and Remembrance Day commemorations.

- Seawall Renewals is underspent by $0.7m to date and forecast to carry forward $1.2m at year end. Most of this is for Ngunguru Seawall Stage 2 which is undergoing a review based on community-driven aspirations, and the balance is for Matapouri.

- Sports & Recreation LOS is forecast to be underspent and carry forward $1.9m at year end for the Pohe Island Development made up of $1.1m for the central carpark and $0.8m for the eastern carpark to align with Bike Northland's proposed development.

- The New Airport Evaluation project is underspent and forecast to carry forward $1.3m due to further CAA investigations and planning matters.

Carry Forwards Update

The carry forwards report (attachment 2) details the infrastructure projects current forecast carry forwards, as well as likelihood of the worst-case scenario.

Significant carry forwards include:

- $2.7m for the LED Street light Project due to a delay in the supply of fittings coming from Italy.

- $0.7m for the Ngunguru Seawall which is being reviewed as parts of the community expressed opposition to the plans.

- $0.7m for carparking on Pohe Island to support the proposed Bike Northland development which is currently sourcing funding.

- $1.1m for Pohe Island central carpark construction which will push into the following year once design and consultation has been completed.

- $1.3m for the Airport Evaluation which is currently working through planning and detailed CAA matters.

The forecast carry forwards of $12.3m fall into three main categories as follows:

- $6.7m is due to expenditure being dependent on external parties' project progress, supplier delays, and unsuccessful tendering for physical works;

- $1.6m is due to project review based on community feedback, and construction periods programmed to allow public use during peak season;

- $4.0m is due to programme delays occurring from weather events experienced in January and February, resources, and internal alignment of interdependent projects;

- An additional $1.7m is at risk of further extreme weather or supplier issues as a worst-case scenario.

Expenditure and Carry Forwards Trend

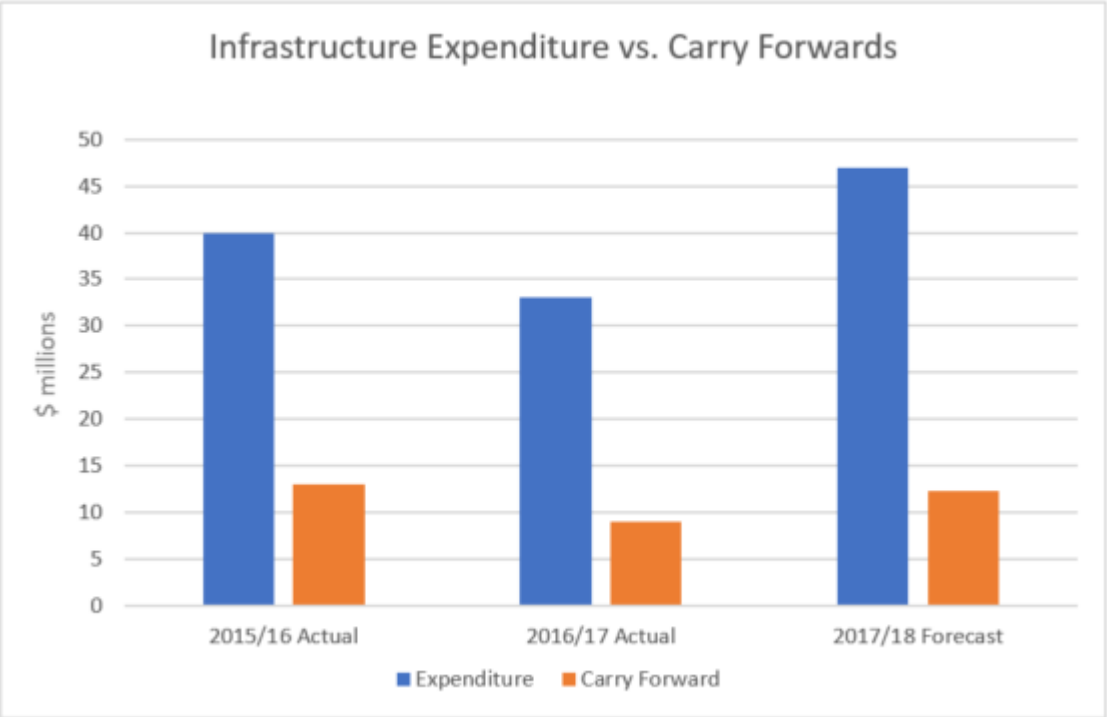
In the 2015/16 financial year end we carried forward $13m after a $40m Infrastructure spend (24% of budget carried forward). Results for the 2016/17 financial year end show a $9m carry forward and a $33m spend (21% of budget carried forward).

The forecast position for end of the current 2017/18 financial year is a $12.3m carry forward, and a $47m spend forecast (21% of budget to be carried forward). Our downside scenario is an $14m carry forward (24% of budget).

The Infrastructure capital works budget is almost 40% higher than it was last year.

The graph in Figure 1 shows the forecast year end position in relation to the previous two financial years.

Figure 1: Infrastructure Expenditure vs. Carry Forwards



5 Significance and engagement

The decisions or matters of this Agenda do not trigger the significance criteria outlined in Council's Significance and Engagement Policy, and the public will be informed via agenda publication on the website.

6 Attachments

1. Capital Projects Report – Infrastructure – March 2018
2. Infrastructure Carry Forwards Report – March 2018

CAPITAL PROJECTS CARRY FORWARDS REPORT

AS AT 31 March 2018

(Figures include both Operating and Capital Expenditure)

LTP Indicator	*Project ID Description*	*March Forecast Carry Forwards*	*Worst Case Carry Forwards*	*Risk*	*Comments*
Cycleways - Programmed Work	Kamo Cycleway - Stage 1 Rust Ave to Kamo Rd	791	791	Medium	Delay due NZ Rail issues. Funding increase to match available subsidy, offset against other savings.
Land for Roads	Land for Roads	500	500	High	Land acquisition negotiations unlikely to be settled by June 2018.
LED Streetlight Upgrades	Streetlights - LED upgrades	2,700	2,700	High	Delay in supply of fittings from Italy.
Lower James Street Upgrade	Lower James St Upgrade	518	518	High	Sense of Place project timeframe aligned to City Centre Plan implementation.
Southern Entrance Intersection Improvement	Design - Southern Entrance	645	645	High	Council contribution by agreement, waiting for developer.
Urban Intersection Upgrades	Porowini/Tarewa Intersection improvements	838	838	High	No suitable tender received. Project needs to be re-tendered 2018/19
	Transportation Total	**5,992**	**5,992**		
Reservoir Rehabilitation - Programmed Work	Reservoir Rehabilitation 2017/18	85	85	High	Works delayed at Water Services request to mitigate risks during 'cyclone season'.
Whau Valley New Water Treatment Plant	New Whau Valley Water Treatment Plant	-	300	High	Award of professional services contract has been delayed 6 weeks.
	Water Total	**85**	**385**		
Transfer Station Upgrades	Rural Transfer Station Upgrades	-	200	Medium	Parua Bay transfer station consent and mediation. Consent application due to be lodged in May. Mitigation works estimated.
	Solid Waste Total	**-**	**200**		
Hikurangi Sewer Network Upgrade	Hikurangi Sewer Network Renewal	-	100	Low	Construction under way, but could fall behind programme.
Wastewater City Service Level Improvements	Tarewa Park Storage Tank	777	894	High	Project under construction with timeframe extended due to adverse weather conditions Carry forward budget to complete construction.
Wastewater City Service Level Improvements	Wastewater Sewer Line and Manhole Renewals	-	376	High	Programme delayed due to staff resignation.
Wastewater City Service Level Improvements	Tarewa Park Trunk Sewer Stage 2 (SH1 Crossing)	-	243	High	WDC funding aligned to NZTA project timeframe.
Wastewater Strategy - Programmed Work	Wastewater Environmental Improvements	32	32	High	Works scheduled with Whitebait Connection.
	Waste Water Total	**809**	**1,645**		
Stormwater Catchment Management Plans & Assessments	Stormwater Catchment Management	100	100	Medium	Morningside assessment
Stormwater Projects - Programmed Work	Marsden City Stormwater	40	40	Medium	
	Storm Water Total	**140**	**140**		
Coastal Structures Renewal	Limestone Island Pontoon Renewal	-	50	Medium	Potential for pontoon construction and installation to be delayed to next financial year.
Hatea Activity Loop	Bascule Car Park Seal	199	199	High	Physical works planned to start in Sept 2018.
Neighbourhood & Public Gardens Renewals	Laurie Hall Park	310	310	High	Physical works planned to start in Nov 2018 to avoid Remembrance Day and ANZAC Day commemorations.
Playgrounds & Skateparks Level of Service	Tikipunga Children's Park	93	93	High	Budget being re-prioritised to Ngunguru Activity Zone. Consulting with the community to confirm scope and location of the project.
Seawalls Renewal	Matapouri Seawall	430	430	High	Concenting and physical works timed to avoid disruption during peak summer season.
Seawalls Renewal	Ngunguru Seawall	749	749	High	Concept re-design under way to align with community feedback, which has extended the project timeframe.
Sport & Recreation Level of Service	Bike Northland Carpark - Pohe Island	747	747	High	WDC funding carried forward to align with 3rd party project timeline.
Sport & Recreation Level of Service	Camera Obscura	80	80	High	WDC funding carried forward to align with 3rd party project timeline.
Sport & Recreation Level of Service	Pohe Island Carpark (Master Plan)	1,087	1,087	High	Design & consultation process pushing construction into 18/19 year. Some fill importation completed this year.
Sport & Recreation Renewals	Otaika Field Renewals	132	430	High	Construction under way but behind programme, lighting installation will be after June.
Town Basin - Conversion of Carpark to Park	Town Basin Conversion of Car Park to Park	149	149	High	Project timeframe extended to align with adjacent Hundertwasser project.
	Community Facilities & Services Total	**3,976**	**4,324**		
New Airport Evaluation	New Airport Evaluation Project	1,327	1,329	High	Currently working through planning matters.
	Support Services Total	**1,327**	**1,329**		
	Total	**12,329**	**14,015**		

INFRASTRUCTURE CAPITAL PROJECTS REPORT

AS AT 31 March 2018

(Figures include both Operating and Capital Expenditure)

	Actual YTD $000	Revised Budget YTD $000	Variance YTD $000	Full Year Forecast $000	Full Year Revised Budget $000	Forecast (Underspent)/ Overspent $000	Forecast Carry Forwards $000	Total (Underspent)/ Overspent $000
Transportation								
Bus Shelters	9	0	9	10	0	10	0	10
Coastal Protection Structures - Roading	0	75	(75)	80	80	0	0	0
Cycleways - Additional government fundin	38	0	38	35	0	35	0	35
Cycleways - Programmed Work	2,363	3,162	(799)	4,274	4,215	58	791	849
Cycleways - Unsubsidised Programmed Work	2	91	(89)	91	91	0	0	0
Drainage Renewals	539	333	206	940	444	496	0	496
Footpaths Renewals	221	255	(34)	340	340	0	0	0
Land for Roads	1,321	500	821	1,331	500	831	500	1,331
LED Streetlight Upgrades	2,533	2,731	(198)	3,900	6,600	(2,700)	2,700	0
Lower James Street Upgrade	0	389	(389)	0	518	(518)	518	0
Mill Rd/Nixon St/Kamo Rd - Roading	23	0	23	50	0	50	0	50
Minor Improvements to Network	1,440	1,956	(517)	3,216	2,608	608	0	608
New Footpaths	176	185	(9)	224	223	1	0	1
Parking Renewals	183	93	90	224	124	100	0	100
Parking Upgrades	0	100	(100)	0	100	(100)	0	(100)
Replacement of Bridges & Other Structures	1	0	1	1	0	1	0	1
Roading Property Renewals & Improvements	16	0	16	16	0	16	0	16
Seal Extensions - House Frontage Sealing	1,157	113	1,044	1,410	151	1,259	0	1,259
Seal Extensions - Wright/McCardle	0	938	(938)	0	1,250	(1,250)	0	(1,250)
Sealed Road Pavement Rehabilitation	3,087	5,130	(2,043)	3,630	6,840	(3,209)	0	(3,209)
Sealed Road Resurfacing	3,732	2,802	930	4,280	3,736	544	0	544
Southern Entrance Intersection Improvement	0	0	0	0	645	(645)	645	0
Structures Component Replacement	369	145	224	802	429	372	0	372
Traffic Sign & Signal Renewals	439	397	41	825	529	296	0	296
Transport Planning Studies & Strategies	47	0	47	65	0	65	0	65
Unsealed Road Metalling	470	827	(357)	1,000	1,102	(102)	0	(102)
Urban Intersection Upgrades	191	816	(625)	250	1,088	(838)	838	(0)
Transportion Total	**18,357**	**21,037**	**(2,680)**	**26,993**	**31,615**	**(4,622)**	**5,992**	**1,370**

Water								
Minor Projects - Emergency Works	22	190	(168)	249	315	(67)	0	(67)
Pipeline Bridges - Programmed Work	0	21	(21)	21	21	0	0	0
Reservoir Rehabilitation - Programmed Work	68	315	(247)	226	315	(90)	85	(5)
Reticulation - Programmed Work	405	542	(137)	833	654	179	0	179
Ruddells Raw Water Line Renewal	273	1,052	(778)	640	1,052	(412)	0	(412)
Water Meter Renewals	354	368	(14)	341	368	(27)	0	(27)
Water Treatment Plant & Equipment Replacement	297	244	53	491	315	176	0	176
Water Treatment Plant and Equipment Replacement	0	0	0	0	0	0	0	0
Whau Valley New Water Treatment Plant	601	1,150	(549)	1,626	1,500	126	0	126
Water Total	**2,020**	**3,882**	**(1,862)**	**4,427**	**4,541**	**(115)**	**85**	**(30)**
Solid Waste								
Transfer Station Upgrades	4	200	(197)	200	200	0	0	0
Solid Waste Total	**4**	**200**	**(197)**	**200**	**200**	**0**	**0**	**0**
Wastewater								
Hikurangi Sewer Network Upgrade	617	858	(242)	960	960	(0)	0	(0)
Laboratory Equipment Renewals & Upgrades	0	34	(34)	34	34	0	0	0
Motor Starter Assessment & Upgrades	0	16	(16)	13	16	(2)	0	(2)
Public Toilets	57	0	57	(42)	0	(42)	0	(42)
Pump Station Upgrades	235	70	165	718	368	350	0	350
Ruakaka Waste Water Treatment Plant Upgrade	72	50	22	465	50	415	0	415
Treatment Plant Remote Monitoring	8	53	(45)	33	53	(20)	0	(20)
Treatment Plant Upgrades	54	504	(450)	340	672	(331)	0	(331)
Tutukaka Wastewater WWTP Renewals	0	33	(33)	33	53	(20)	0	(20)
Waipu Trunk Main Upgrades	46	56	(9)	51	56	(4)	0	(4)
Wastewater Assessment	0	42	(42)	17	42	(25)	0	(25)
Wastewater City Service Level Improvements	2,622	4,540	(1,918)	4,941	5,992	(1,051)	777	(274)
Wastewater Projects	0	200	(200)	(0)	200	(200)	0	(200)
Wastewater Strategy - Programmed Work	29	70	(41)	60	100	(40)	32	(8)
Wastewater Structures Earthquake checks	0	0	0	0	32	(32)	0	(32)
Wastewater Treatment Plants	0	0	0	0	0	0	0	0
Whangarei City Wastewater - Projects	0	0	0	0	0	0	0	0
Wastewater Total	**3,741**	**6,524**	**(2,784)**	**7,623**	**8,626**	**(1,002)**	**809**	**(194)**
Stormwater								
Stormwater Catchment Management Plans & Assessme	0	126	(126)	26	126	(100)	100	0
Stormwater Projects - Programmed Work	374	655	(281)	908	934	(26)	40	14
Stormwater Total	**374**	**781**	**(407)**	**934**	**1,060**	**(126)**	**140**	**14**

Community Facilities & Services								
Parks & Recreation								
Bank Street Revitalisation	0	0	0	0	0	0	0	0
Cemeteries Level of Service	0	75	(75)	25	85	(60)	0	(60)
Cemeteries Renewals	61	120	(59)	157	160	(4)	0	(4)
Coastal Structures Renewal	222	344	(122)	317	364	(47)	0	(47)
Emerald Necklace - Sense of Place	160	215	(55)	173	215	(42)	0	(42)
Hatea Activity Loop	376	472	(97)	787	863	(76)	199	123
Neighbourhood & Public Gardens Level of Service	8	176	(168)	206	176	30	0	30
Neighbourhood & Public Gardens Renewals	443	950	(507)	682	1,123	(440)	310	(130)
Parks Interpretation Information	4	20	(16)	37	42	(5)	0	(5)
Playgrounds & Skateparks Level of Service	(43)	55	(98)	167	260	(93)	93	(0)
Playgrounds & Skateparks Renewals	78	157	(79)	20	167	(147)	0	(147)
Public Art	19	20	(1)	42	42	0	0	0
Seawalls Renewal	638	1,380	(742)	1,014	2,027	(1,013)	1,179	165
Sport & Recreation Level of Service	535	1,341	(805)	917	2,850	(1,932)	1,915	(18)
Sport & Recreation Renewals	841	1,425	(584)	1,405	1,538	(133)	132	(1)
Town Basin - Conversion of Carpark to Park	4	105	(101)	4	203	(199)	149	(49)
Urban Design - Themed Communities & Settlements	69	139	(70)	124	190	(66)	0	(66)
Walkway & Track Level of Service	18	16	2	29	21	8	0	8
Walkway & Track Renewals	269	248	21	585	427	158	0	158
Parks & Recreation Total	**3,702**	**7,258**	**(3,556)**	**6,692**	**10,752**	**(4,060)**	**3,976**	**(84)**
Community Facilities & Services Total	**3,702**	**7,258**	**(3,556)**	**6,692**	**10,752**	**(4,060)**	**3,976**	**(84)**
Support Services								
Infrastructure Planning & Capital Works								
New Airport Evaluation	73	40	33	193	1,520	(1,327)	1,327	(0)
Infrastructure Planning & Capital Works Total	**73**	**40**	**33**	**193**	**1,520**	**(1,327)**	**1,327**	**(0)**
Support Services Total	**73**	**40**	**33**	**193**	**1,520**	**(1,327)**	**1,327**	**(0)**
Total	**28,270**	**39,722**	**(11,452)**	**47,061**	**58,314**	**(11,253)**	**12,329**	**1,077**

WHANGAREI DISTRICT COUNCIL

5.3 Infrastructure Operations Report – May 2018

Meeting: Infrastructure Committee
Date of meeting: 10 May 2018
Reporting officer: Simon Weston (General Manager Infrastructure)

1 Purpose

To provide a brief overview of work occurring in the current financial year across services that the Infrastructure Committee is responsible for.

2 Recommendation

That the Infrastructure Committee notes the Infrastructure Operations Report April 2018 update

3 Background

In December 2016, Council adopted committee terms of reference for the 2016 – 2019 triennium, with the purpose of the Infrastructure Committee being to 'oversee the management of Council's infrastructural assets, utility services and public facilities'.

This report provides the Committee with a brief summary of the operational highlights from the Infrastructure Monthly Report, March 2018.

4 Significance and engagement

The decisions or matters of this report do not trigger the significance criteria outlined in Council's Significance and Engagement Policy, and the public will be informed via report publication.

5 Attachments

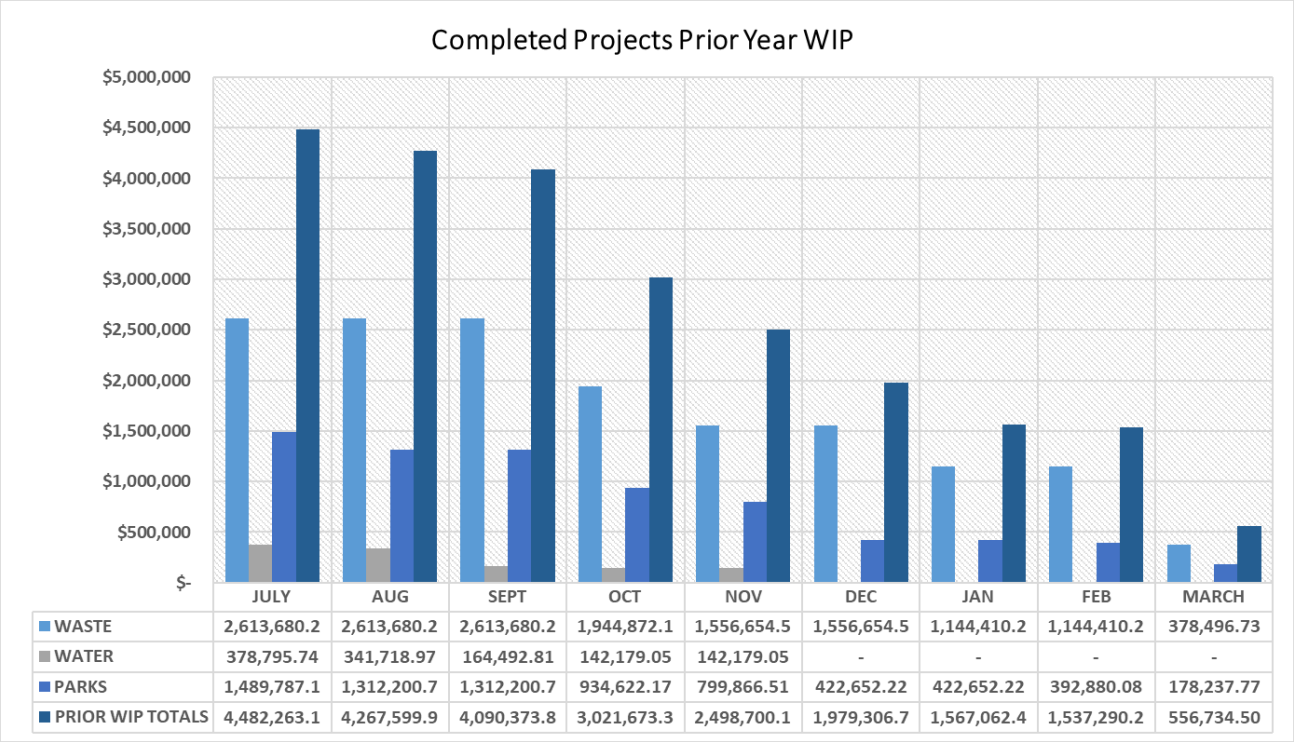
Infrastructure Operations Report – May 2018

Infrastructure Operations Report – May 2018

Infrastructure Development

Capitalisation

Projects from Parks, Water and Waste and Drainage enter the work in progress account (WIP) upon commencement and are removed to Hansen Clearing once closed and documentation required for capitalisation has been provided. The prior period WIP had significant movement in March as staff worked through some of the larger, more complex, projects. To assist this process, and ensure that all prior period WIP is capitalised by year end, one on one sessions have been arranged with the Department Managers and staff responsible for capitalisation of each activity. This resulted in a $1.01 million reduction, the breakdown of which is provided below:



	JULY	AUG	SEPT	OCT	NOV	DEC	JAN	FEB	MARCH
WASTE	2,613,680.2	2,613,680.2	2,613,680.2	1,944,872.1	1,556,654.5	1,556,654.5	1,144,410.2	1,144,410.2	378,496.73
WATER	378,795.74	341,718.97	164,492.81	142,179.05	142,179.05	-	-	-	-
PARKS	1,489,787.1	1,312,200.7	1,312,200.7	934,622.17	799,866.51	422,652.22	422,652.22	392,880.08	178,237.77
PRIOR WIP TOTALS	4,482,263.1	4,267,599.9	4,090,373.8	3,021,673.3	2,498,700.1	1,979,306.7	1,567,062.4	1,537,290.2	556,734.50

The team are also working alongside Finance, who are planning a new round of joint capitalisation training for April. Since July the balance of prior period WIP has reduced by $3.93 million.

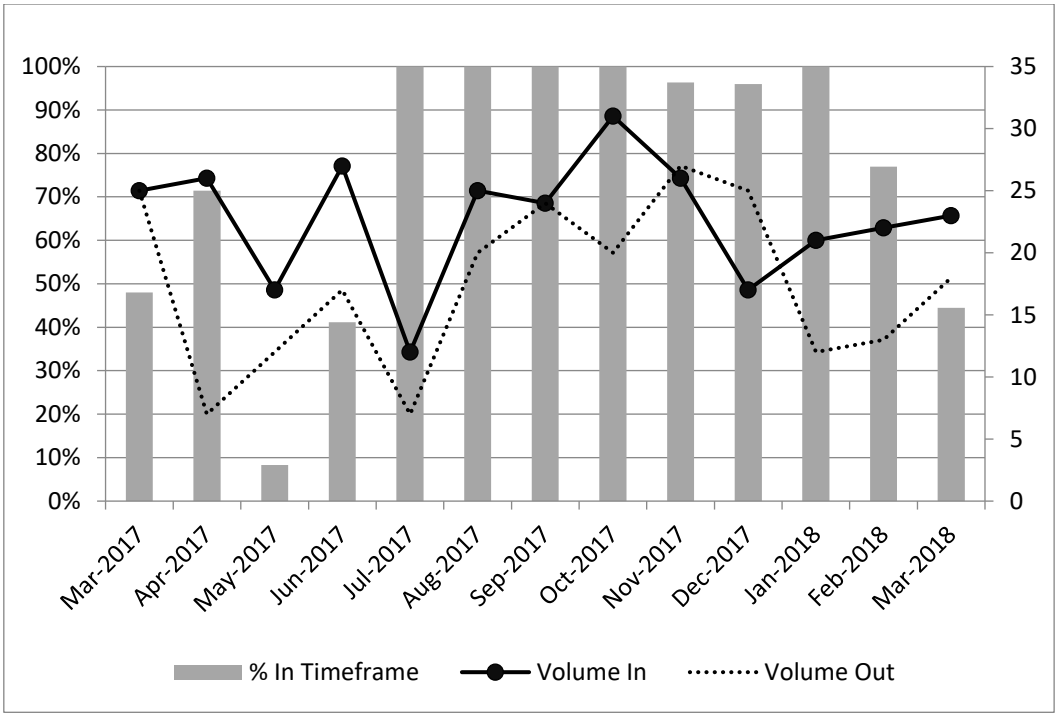
Asset Management and the Long-Term Plan

One of the key projects currently underway for the team is the upgrade or replacement of our Asset Management System, the current version of which is no longer supported. While driven through the ICT department the project has significant input from the asset data and asset management teams who are ultimately the business owners for the system. Over March staff from across Council worked through requirements for the new system in preparation for a request for proposal process.

In March consultation commenced for both the 2018 Long Term Plan and Development Contributions policies, with the team supporting public meetings and providing additional information to the public as required. It is anticipated that April will be a busy time for asset managers as they work through submissions received and any changes that Council wishes to include in the Long-Term Plan as a result of those.

Development Engineering

The Development Engineers shifted to Infrastructure when the new structure came into effect on 01 July. This coincided with the Team filling the vacant Development Engineer's position, which in turn assisted in 100% of applications being processed within timeframe from July to October.



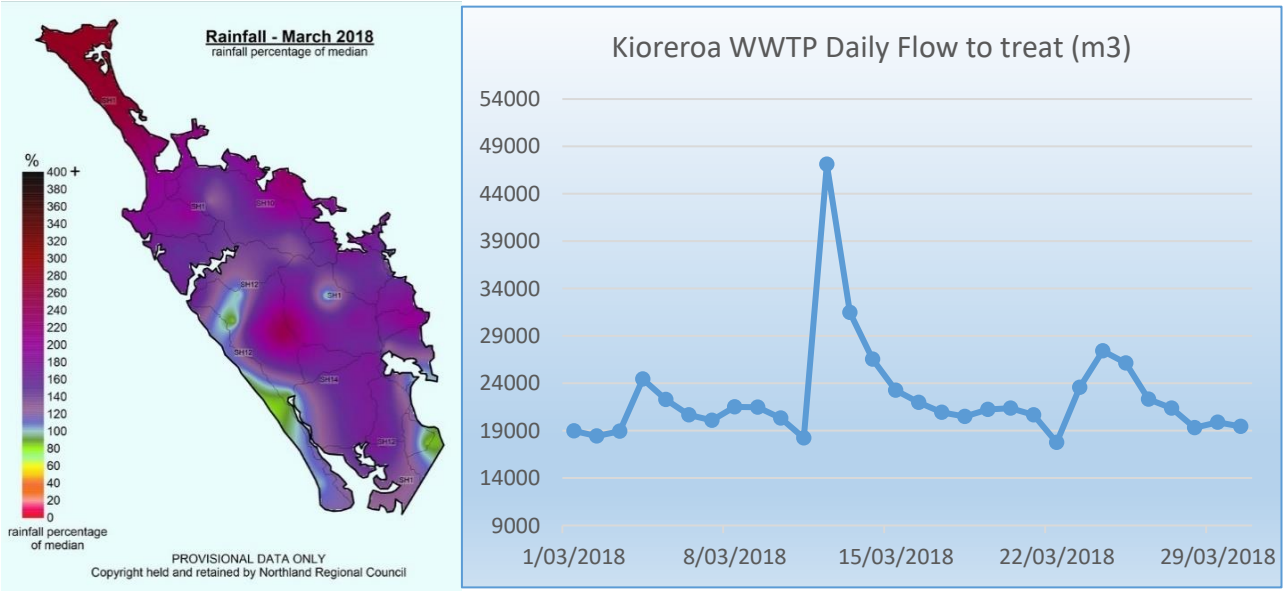
However, the volume of applications received remains high and while applications processed within timeframe returned to 100% January (having dropped to 96% in November and December) the team have received a number of larger, more complex applications in February and March. As a result, applications processed within timeframe dropped markedly last month.

It is worth noting that these are internal timeframes for engineering input to consents, rather than the statutory timeframes for processing consents. While only one of the 41 consents completed in March went over the statutory timeframe delays in engineering input can put pressure on other parts of the process. As a result we are working through processes and resourcing options to clear the backlog and meet demand going forward.

Waste and Drainage

Operations

Whangarei WWTP



NRC image depicting the percentage of normal rainfall and graph illustrating the flow in m^3/d through Whangarei WWTP

Kioreroa WWTP
Biogas generator being reconditioned locally for install in April. The NaOH IBC's are ready for collection as dosing is no longer required at WWTP. A 670KVA generator is being priced for the WWTP. The Digester mixer 1 rebuild was completed. Waipu Primary School completed a tour of the WWTP.

Rural WWTP
Hikurangi membrane filter still inoperable, awaiting report on ability to clean and rectify, this will take 3-4 weeks. Looking at alternatives including UV treatment and changing solids level in consent. The new 400KVA Okara generator was delivered and installed.

Human Resources
Judith Ellensohn started as Project Engineer 3 April taking over Fiona Pratt's position. Judith has a strong background in development inspections and asset management.

Reticulation
There was one sewerage spill in March; which is pleasing after February's five spills.

Date Spill Started / Ceased	Location	Cause	Volume (m^3)	NHDB Notified of the event	Type of Sewage	Action Taken
24/03/2018	Langs Beach bridge by Wairahi Road	Split in the PVC rising main	<2m^3	Yes	Raw/unscreened	NRC and DHB notified, 2 spill signs placed either side of the stream outlet on beach

Capital Works Projects

Sewer CAPEX
Hikurangi Union Street sewer project quoted at $220K hoped to be completed by June 2018. This includes a sewer main replacement and 3 pressure sewer pump stations. This will alleviate 3 properties being unable to use their toilets in Stormwater events and the bad PR associated with them needing portaloos during these events.

Sewer and Stormwater Renewals
Bank/Manse Street storm-water replacement $141K completed. The urgent 101 to 111 Morningside job for road replacement was completed by Hydrotech.

Consents and Compliance
Ngunguru ammonia is still non-compliant with consent conditions.
Hikurangi is non-compliant around disinfection and solids due to membrane problems, we are awaiting report on the membrane and considering alternative options. The laboratory will increase the sampling runs.

Policy and Procedures
Stormwater

A list of proposed Stormwater projects have been put to Hydrotech for quotation. Approved projects will be reported next month.

Hikurangi Swamp Flood Management Scheme
Drain clearing for March as agreed with the Swamp Working Group is detailed below. Helicopter spraying for the year has been deferred as per the direction at the last meeting.

Hikurangi Swamp Drain Cleaning 2017-2018

WO #	Area	Equipment/Item	Description	Remedy	Hansen Code	Distance	Rate	Cost	Date Added
1380512	Luptons Point Drain	Long Reach Digger	Weed Clearing	Weed Clearing	HB9400	3812.0	$ 2.61	$ 9,959.73	31/03/2018
1380512	Otonga Stream	Long Reach Digger	Weed Clearing	Weed Clearing	HB9400	500.0	$ 2.61	$ 1,306.37	31/03/2018
1380512	April Drain	Long Reach Digger	Weed Clearing	Weed Clearing	HB9400	500.0	$ 2.61	$ 1,306.37	31/03/2018
1380512	Wilsonville Quarry Drain	Long Reach Digger	Weed Clearing	Weed Clearing	HB9400	1481.0	$ 2.61	$ 3,869.45	31/03/2018
1380515	Lister Drain	Land Base Spraying	Spraying	Spraying	HB9300	0.8	$ 224.50	$ 178.48	31/03/2018
1380515	Bell Drain	Land Base Spraying	Spraying	Spraying	HB9300	1.2	$ 224.50	$ 274.11	31/03/2018
1380515	Palmer Drain	Land Base Spraying	Spraying	Spraying	HB9300	1.6	$ 224.50	$ 368.63	31/03/2018
1380515	Rushbrook	Land Base Spraying	Spraying	Spraying	HB9300	1.7	$ 224.50	$ 380.75	31/03/2018
1380515	McKenzie	Land Base Spraying	Spraying	Spraying	HB9300	1.4	$ 224.50	$ 309.14	31/03/2018
			Total	Kms		6299.7		$ 17,946.06	

Te Mata and Okarika flood gates to be repaired. Ongoing discussions with IWI and NRC over eels deaths at Hikurangi pump stations due to pumps and low oxygen water.

Solid Waste Operations

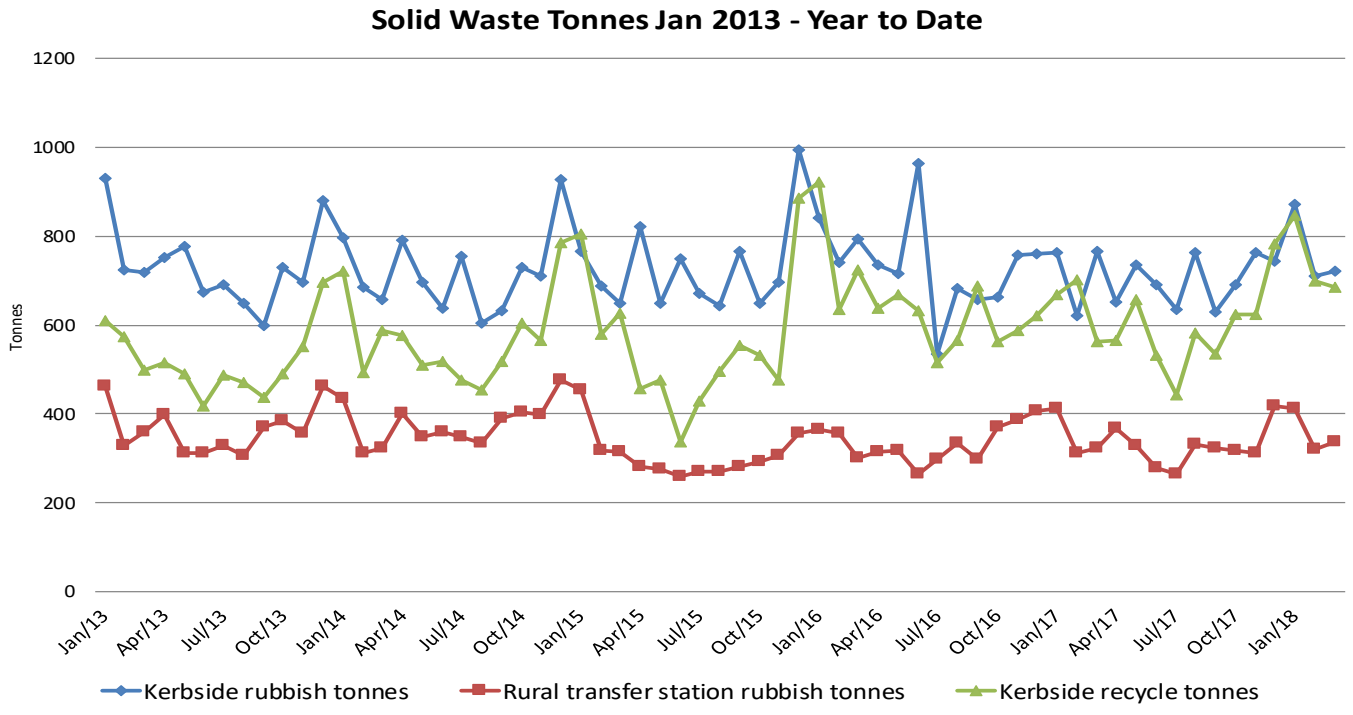
The receiver of kerbside mixed recyclables will no longer take material from Whangarei District Council collections due to market changes and a fire at their processing facility. Alternative processing options are being explored.

Monthly waste tonnages are shown in the table and charts below.

Rubbish Collection Tonnes	2017/18	2016/17	RTS Tonnes	2017/18	2016/17	Recycling Tonnes	2017/18	2016/17
June		690	June		277	June		532
May		736	May		328	May		658
April		653	April		368	April		566
March	722	766	March	335	321	March	685	561
February	711	622	February	320	311	February	700	701
January	873	762	January	412	412	January	848	668
December	742	760	December	439	406	December	783	621
November	764	758	November	311	386	November	624	589
October	690	663	October	316	371	October	623	563
September	630	656	September	323	298	September	527	688
August	764	681	August	330	332	August	441	567
July	634	535	July	264	297	July	583	515
Total for period	**5808**	**8282**	**Total for period**	**2715**	**4107**	**Total for period**	**5129**	**7229**

Kerbside Rubbish and Recycling Collection and Rural Transfer Station Operations

Rubbish and recycling and transfer station figures are similar to last year.



Laboratory Report

Production

The Laboratory received 1059 samples requiring 2924 tests during March; 349 tests were subcontracted. 66% of jobs were reported within 5 working days. Sample numbers continue to show an increase compared with last year, current increase percentage of test numbers for the year to date is 12.1%.

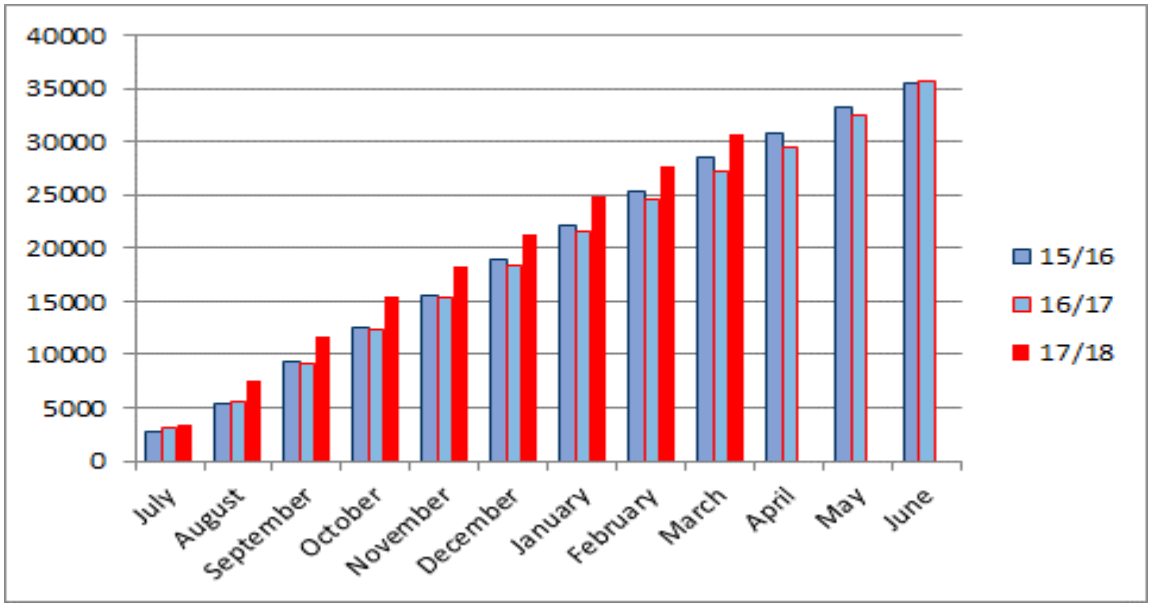


Figure 1; Tests performed to date for current year ending.

Health and Safety

The Laboratory participated in a site health and safety audit with favourable comments by the auditor on the management of health and safety in the laboratory. An identified step hazard outside the Lab entrance has been removed by the installation of a ramp. The excessive heat environment in the workshop area experienced this summer has been removed by installation of air-conditioning plus insulation and lining of the roof.

Equipment and Maintenance

The leaking roof above the equipment room is scheduled to be repaired.

Infrastructure Planning & Capital Works

Major Projects

Sense of Place Projects

- Carpark to Park: Initial project scoping is complete, including concept design and budget estimate. The Car Park to Park Working Party is meeting regularly. A decision has been made to time the physical works so that they do not interfere with the construction of Hundertwasser Art Centre. It is likely that construction of Car Park to Park will need to start in 2019-2020 with completion in late 2020.
- Hatea Loop Lighting: Physical works are complete and lighting commissioned.
- Amenity Dredging: This year's dredging is underway, starting outside the Art Park where last year's work ended.
- Whangarei Entranceways Signage: Civil works for the new Entranceway and Signage at Mander Park was completed in December 2017, and planting was completed in March. The project is now complete.
- Camera Obscura: The external project team are in the process of seeking funding for the construction of a camera obscura on Pohe Island. Preloading of the site with metal was postponed to April. Construction is planned to start in August 2018.

Parks & Recreation Projects

- Matapouri Beach Restoration: Peer review of the engineering design is complete and comments are being addressed. Community consultation will begin in April and the consent application will be lodged. It is anticipated that replenishment works start in spring 2018.
- One Tree Point Cliff Erosion Management: This project is on hold until a coastal erosion protection strategy along the whole of One Tree Point has been completed.
- One Tree Point Seawall Investigation: The investigation will look at the entire length of coast around the One Tree Point area and will be the first step to develop a consistent approach to erosion management in the area. The professional services tender was awarded in January to RS Engineering and the final report is due in June 2018.
- Seawall Renewals 17/18: The design and consenting works have been awarded to Hawthorn Geddes and are underway with the consent applications submitted to NRC.

- Ngunguru Seawall Renewal: Stage 1 at Te Maika Road is completed with agreed remedial works to the top of the wall and grass area to be completed once the contractor is available. Stage 2 seawall renewals along Ngunguru Road frontage are being reviewed based on community feedback. A concept plan is being prepared to refine the scope of work. Cost estimates will then be compared to budget. Additional funding may be necessary to achieve the community's desired outcomes.
- Sandy Bay Beach Restoration: An investigation is underway to determine a long-term solution to manage erosion. Once this has been completed, a concept plan will be presented to the community for consultation. This plan will include dune protection areas.
- Otaika Sports Park Field Construction: Construction of two new sports fields, including lighting, irrigation and drainage is currently underway, and the fields will be ready for the winter season April 2019.
- Hikurangi Multiuse Hardcourt: Construction of the hardcourt is complete with only minor finishing works required. Resource Consent to install the lighting has been approved.
- William Fraser Memorial Park Development Pohe Island: Professional Services have been awarded to Hawthorn Geddes to design and document the central island carpark design through to physical works. Proof of concept is complete and review comments returned for inclusion into the detailed design. Bulk fill importation has begun to provide a stable sub-grade for pavement works.
- Ruakaka Sports Fields: The design for the two new fields and refurbishment of the existing fields is nearly completed. A Registration of Interest for the sand removal has begun and is due to close 9 April 2018.
- Laurie Hall Park: Drainage works are complete. Design for stage 2 of the upgrade of Laurie Hall Park is under revision. Construction of the new pathways and lighting is planned to start in the 2018/19 financial year.
- Parihaka Track Renewals: The Contract has been awarded to Plantpro & Sons. The Drummond Track was completed and open the Thursday before Easter weekend. Work on the Dobbie / Hokianga track started immediately after Easter.
- Limestone Island Pontoon – RS Engineering has been engaged to complete the design of the pontoon.
- Abbey Caves Car Park and Toilet Facility - The feasibility study for this facility has been awarded to Hawthorne Geddes. Preliminary concept plans are due to Council at the beginning of May. Final concept plans and a report will be completed by the beginning of June.
- Quarry Gardens Car Parking Facility – A consultant has been engaged to undertake a topographical survey and provide a feasibility study to increase the number of car parks available at the Quarry Gardens, consider the safety of pedestrians and investigate the feasibility of providing access and parking for buses.

Water Projects

- New Whau Valley Water Treatment Plant: The preferred treatment process has been selected. Preliminary design is due to be completed in April 2018. Negotiations for the detailed design contract are currently underway.
- Ridermain Replacements 2018/19: The in-house design for various work packages is currently underway, and planned to be completed by May 2018, followed by the tender period and construction in 2018/19.
- Three Mile Bush Reservoir: Previous site location studies are currently being assessed and validated, with additional site investigation work to be performed to inform site selection process.
- Reservoir Rehabilitation 2017/18: Contract awarded to Steve Bowling Contractors. Works are complete at the Onerahi and Parua Bay Reservoirs. Work at Ruakaka and Ruddells has been delayed until after cyclone season. Completion is planned for May 2018.

Waste & Drainage Projects

- Tarewa Storage and Emergency Overflow Tank: Construction is currently underway and is due to be completed July 2018 with commissioning work in August. Extremely wet weather during January and February has reduced the number of days on site and consequently extended the completion date.

Planning & Regulatory Projects

- New Animal Shelter: This project is currently being scoped by the sponsor. Concept pans followed by a feasibility study to determine a suitable site will be undertaken during the 2018/19 financial year. This work will be tendered in July 2018.

Infrastructure Planning

The team is continuing to work with Policy in the comprehensive review of designations held by WDC. This requires research on the history and function of each designation to establish whether it has been given effect to. In many cases, corrections or changes to the designation are needed to provide for the effective ongoing operation of the facility.

Parks and Recreation

Operational updates

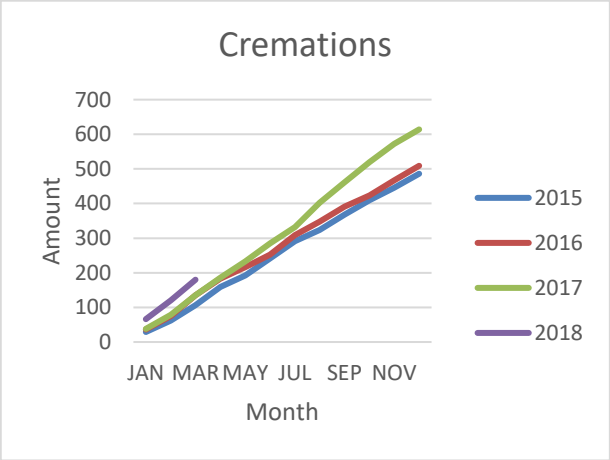
Tracks:

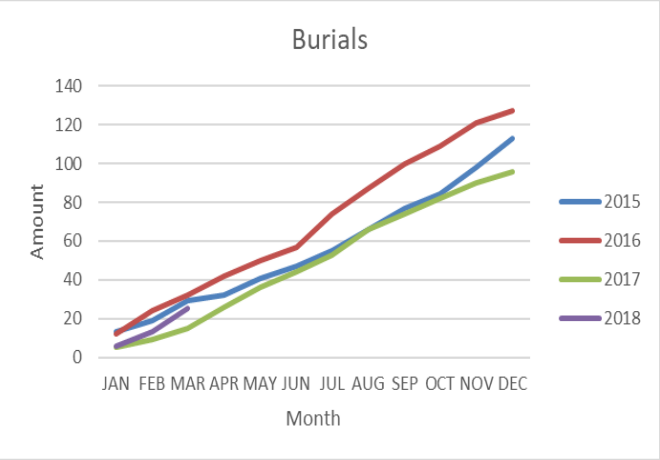
The upgrade of the Drummond Track was completed in time for Easter. The upgrade has resulted in the entire length of the track being resurfaced so that it is consistent for users and the new steps are much easier to walk. Plantpro have started work on the adjacent Dobbie Track.

March concludes the maintenance contract CON11014 held by Northland Parkcare Ltd for the past 5 years. Northland Parkcare will continue the maintenance of our walking tracks under CON17033 from 1st April. Further new routes will be adopted from that date including the Waipu Coastal Walkway (this has been walked and recorded with a lot of improvement work needed) and the track between Ngunguru and Whangaumu.

Cemetery:

The cremation numbers continue to rise compared with previous years with a spike due to the Kerikeri crematorium being closed for maintenance and the Maunu facility undertaking all Northland cremations. Burial numbers to date are higher than last year although a bit earlier to confirm a trend.

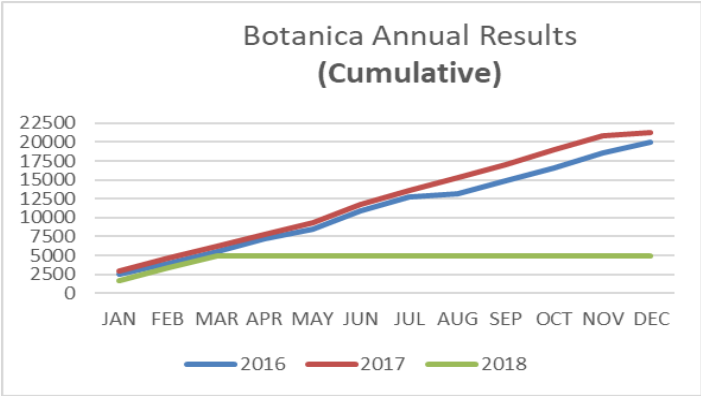




At Maunu cemetery the Returned Servicemen flag pole has a bend as a result of some recent earlier repairs. There is no immediate danger of it falling over however it is noticeable. This has been discussed with the President of Whangarei RSA and work is planned after ANZAC Day to avoid creating a mess of the grounds. In the meantime, a strop has been attached to anchor it and stop it leaning any further.

Botanica

Visitor numbers are consistent with previous year. The first wedding in long time was held at the premises as well as a visit from a local walking group and students from Northtec which kept the staff busy. Due to falling fronds a number of palms will be removed.



Trees:

Treescape having been gearing up throughout March for the start of their new maintenance contract commencing 1st April – CON17031. There are now two crews working in the District, one dedicated to the road and reserve tree maintenance and the other undertaking clearance work on behalf of Northpower and undertaking any additional council requests.

Asset data is being transferred to a new asset management system 'Tree Cloud'. This provides live records, updates GIS maps and forms the basis for invoicing under the measure and value rates agreed within the contract.

Work has been undertaken within Kensington Park and Bush Haven Reserve plus some work on Council owned listed heritage trees. Several 'Cut or Trim Notice' jobs provided by Northpower have also been completed. CRM requests were still high this month at around 50, due to poor weather earlier in the month.

Treescape's internal trainer has been in Whangarei for two weeks this month completing competence assessments on new staff members and getting one of them up to speed with working around the power lines.

Coastal Structures:

The first briefing from RS Consulting on the findings of the coastal protection options report for the coastline from Pyle Road West to Marsden Cove were held. In the meantime, interim repair measures are nearly complete along the timber retaining wall near Pyle Road West. The Request for Price (RFP) to remove the illegal seawall constructed of pipes has closed with a price of $15,000 received. This work is planned for April, subject to tides.

As reported previously, repair options for the Limestone Island pontoon are underway. However, the ranger has said they can manage by using the northern lagoon and boat for another six months to disembark visitors. Unfortunately, there are limited providers of marine services and the outcome of discussions with suppliers is that the work will not be completed until September 2018. Although some work to build the pontoon, off shore, will start this finacial year.

A RFP to undertake a condition assessment of all coastal structures, excluding any in the One Tree Point project, was won by OPUS International Consultants. The condition report is due late April and will form the basis of the maintenance contract tender. This maintenance tender is due to be released mid-May with a view towards the new contract starting 01st August 2018.

Sports fields:

Significant rain fell during March, and with the hot humid conditions, grass growth has been strong again. Ground conditions are wet and are getting similar to those normally well in to winter time.

Further action at Cobham Oval kept ground staff very busy with two Plunket Shield matches held during the month.

The changeover from summer to winter codes is continuing, with most fields being prepared for winter codes.

A spate of padlock vandalism at Otaika Sportspark has led staff to engage the services of a covert surveillance camera to try to catch whoever is doing this vandalism.

The contractor has appointed a new Head Groundsman, who will start mid-May.

Audit results averaged 93.75% against a target of 90%.

Parks and Gardens:

The contractor continues to stain, paint and maintaining Parks assets. These included numerous seats, signs, posts and picnic tables. The rotten boards along the boardwalk in the Town Basin have been replaced.

Assets from Urquharts Bay through to McLeods Bay to Reotahi, Bay View, Mt Manaia and Taurikura as well as Scow Landing, Matapouri and Sandy Bay were checked and inspected.

Numerous assets were cleaned and water blasted in March, including signage at Nixon St Reserve and Scow Landing as well as the Memorial at Scow Landing. The boat ramps at Parua Bay and One Tree Point were water blasted in preparation for a busy Easter weekend at the end of the month.

With a few heavy rainfall periods, numerous drains got checked and cleared. These included Mair Park, Hodges Park, Tait St Reserve and Laurie Hall.

March saw an upgrade to the area around the Riverside theatre. The old fence around the site has been replaced with new bollards. The carpark was resealed and line marked, and a new sign has been placed at the entrance to the carpark stating that it is open to public parking. The sign can be flipped over to display private parking for events at the theatre.

Playgrounds and Skate parks:

During March, the contractor had to clean up vomit at Ngunguru playground, faeces at Kensington playground and dog poo at Tarewa playground. Very little other damage or issues were encountered during the month and the contractor concentrated on rust prevention and playground cleanliness.

Ruakaka playground – renewal

General news

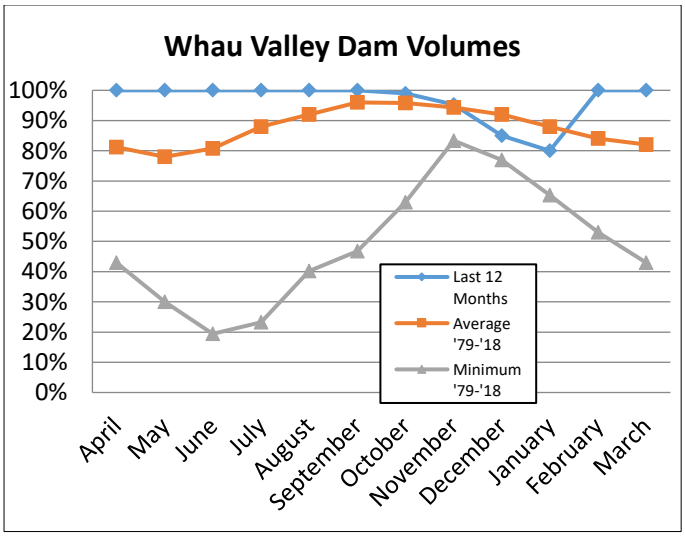
Work is underway to develop a record of community owned buildings (regional, district and local facilities) and details of any cyclical maintenance plans that they may hold. The purpose of this project is to work with these assets owners to assist them to develop these plans so that community buildings are well maintained and Council is not approached last minute with request for funds due to failure of these buildings i.e. immediate roof replacement required. This is an on-going project and may influence the success or not of Council grant funds in the future.

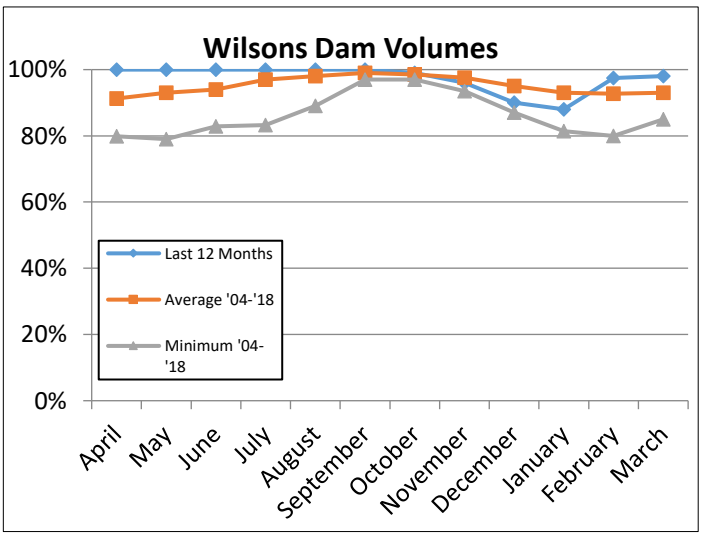
Back in 2015 Council received a draft policy General Objectives and Policies for Council administered Reserves and resolved to seek comment from Te Huinga, sports clubs, and other interested parties. Planning is underway on how to seek this feedback. Once it is received it will be summarized and reported to the Committee.

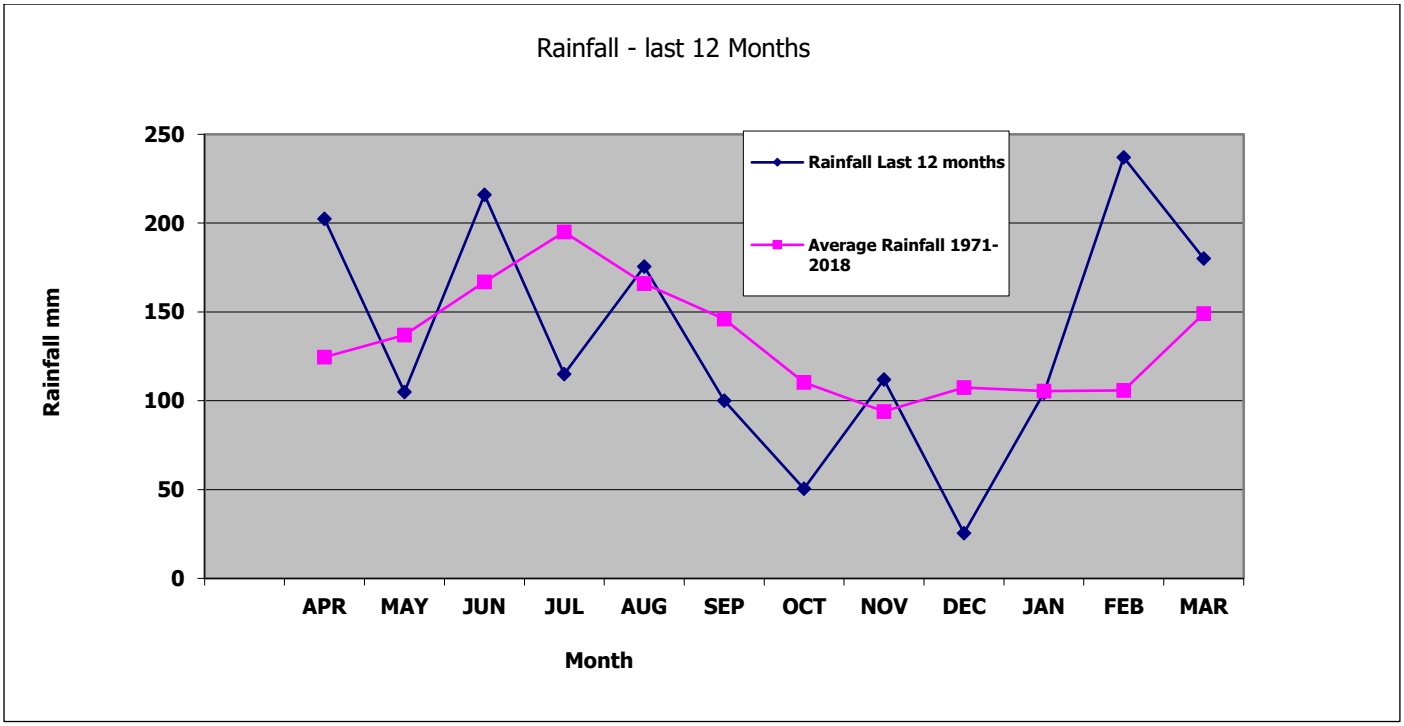
Water Services

Rainfall and Water Sources

March was another wetter than average monthly with 180mm of rain compared to the normal 149mm. After a quarter of the year we have had 160mm more rainfall than normal. Luckily most of the rainfall has been evenly spread and has not caused too many problems with the catchments or treatment plants. Both dams remain near 100% full and rivers have more water than normal for the time of year. The long range forecast is for more rain heading into winter.







Production Report

Treatment plants ran well during March with callouts reduced from previous months and no major incidents. The Algal bloom at Wilsons Dam persists and the Ruakaka plant continues to struggle to cope with the volumes of sludge produced. A project has been started to put scrappers into the clarifiers at Ruakaka which should assist with the overall performance of the plant. Whau Valley Dam is also experiencing a bloom although nowhere near the extent of Wilsons. However, if the bloom persists once the Dam is reopened after logging signage maybe required to inform the public.

Capital Works

Work is nearly complete on the preliminary design for the new Whau Valley Water Treatment Plant. This will be followed by the detailed design before the work can be tendered. Work is nearly complete on the logging of the pine trees at Whau Valley. It will take a further month to tidy up the site before it can be reopened to the public.

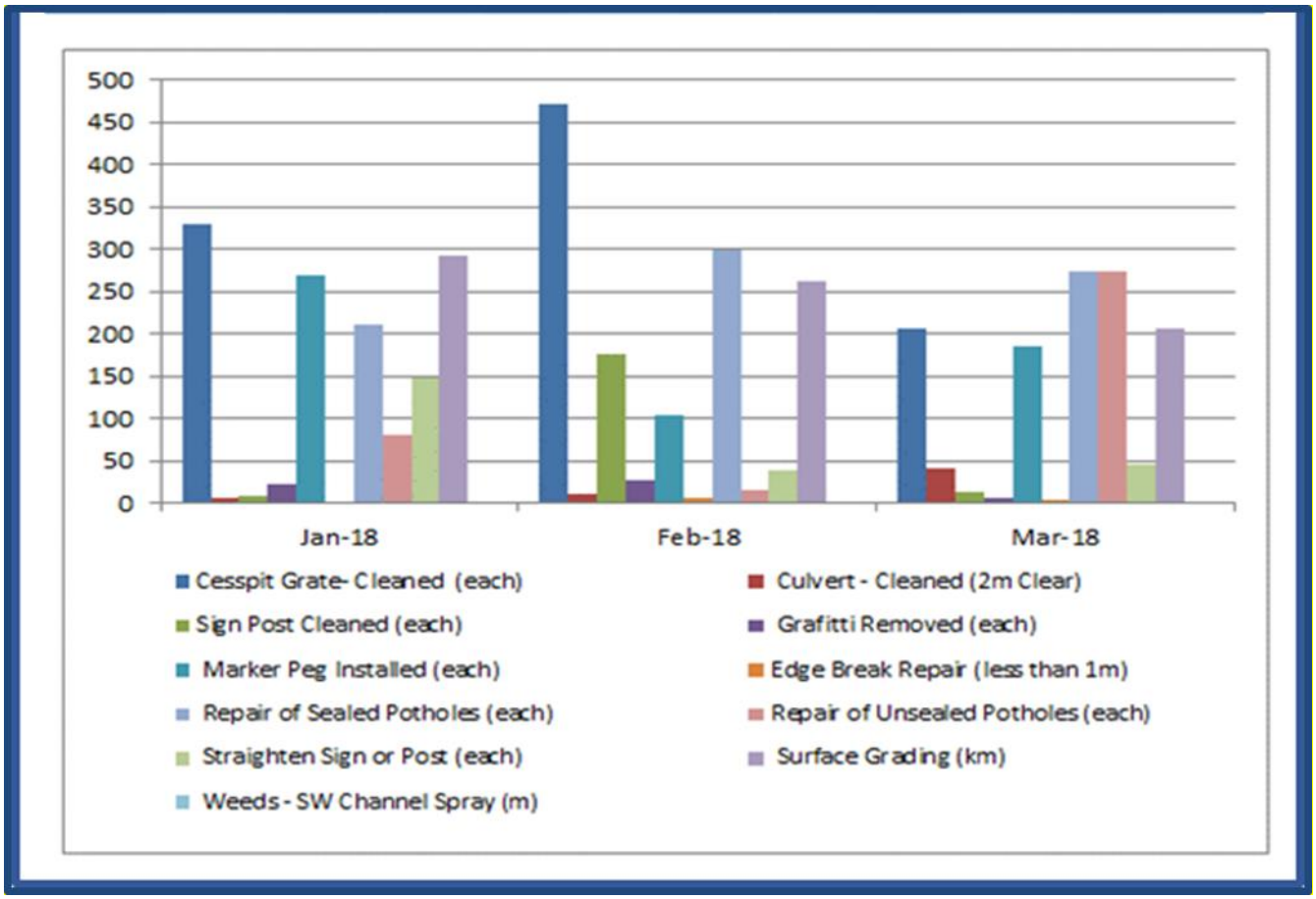
Work on the meter replacement is now complete for the year. The Ruddells raw water line replacement project tendered before Christmas was awarded in mid-February. Work is now well underway and all the pipe should be in the ground by the end of April. Connections to the existing lines will then take several weeks. The main replacement contract is also underway with mains being replaced in Albany Road, Tikorangi Place and Kent Road. Work is expected to be complete by the middle of June.

Roading

Maintenance

Routine work this month has been mainly responsive work due to adverse weather conditions for this time of year involving pothole repairs, surface detritus, cleaning and maintenance grading. Two dedicated patrol units have been concentrating on routine work including general maintenance of signage and other assets, edge marker pegs, minor vegetation sight clearing and cesspit grate/surface water channel cleaning.

Roading Operational Outputs - **Monthly Achievement – Routine Works**



Pavement Rehabilitation and Seal Extensions.

The programme of rehabs on rural roads has commenced and is programmed to be all completed by April this year. Works have been completed on 3 small sections on Kokopu Rd, as well as a major slip repair on Abbey Caves Rd. A section on Springfield Rd, and Rehabs on Pipiwai Rd and Whatitiri Rd are currently under construction.
The seal extension contract for 2.5km of new seal on Wrights and McCardle Rds has been awarded and construction started in early late November. 4 of the 6 sections of road to be sealed as part of this project are complete and work has commenced on the final 2 sections.

Springfield Road Rehab.

Wright Roadd Seal Extension

Bridge Repair

The upgrade of the first of the bridges on Doctors Hill Road and the Thompson and Old Tokatoka Rd bridges are now complete. Upgrade of the second bridge on Doctors Hill Road, Ararua Rd and Wilson Rd bridges will be completed by April of this year. Major bridge maintenance contracts for scour protection and general maintenance are under construction.

Doctors Hill Bridge Repair

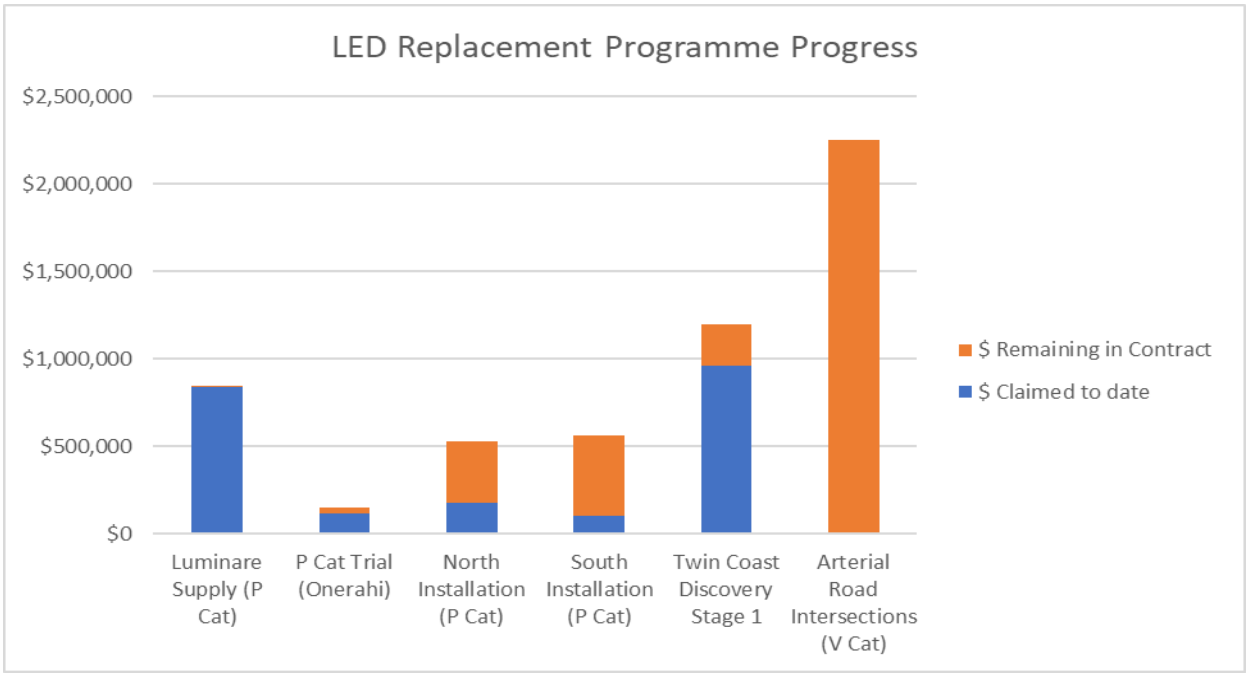
LED Street Light Replacement

LED Street Light Conversion

Funding has been approved from NZTA for $6.6M to replace the existing streetlights with energy efficient LEDs. This work was funded at 85% FAR (subsidy) for work completed by 30 June 2018. NZTA have just announced that the 85% subsidy rate has been extended up until June 2021

The installation trial that was undertaken on the Onerahi/Whangarei Heads area is now complete.
The two contracts for the installation of the P-Category lights in the northern and southern halves of the Whangarei District Council area have been awarded to Currie Electrical and McKays. Work on these contracts started in January and is due to be completed in May. In total, 1,730 P-Category lights (52%) have been installed to date.

The upgrade of the V-Category (Arterial road) lights on the Twin Coast Discovery Highway in Whangarei City, which is being undertaken by Currie Electrical, is nearing completion.
The V Category lights for the arterial road intersections have been designed and the luminaires ordered. These luminaires are expected to arrive in June. As there are only 83 intersection lights, this work is likely to be included in the Twin Coast Discovery Highway contract as a variation.



The 200 V-Category lights in rural areas will be designed by early March and it is expected that these should be ordered and installed before the end of June. The remaining 1,000 V-Category lights for mid-block areas in Whangarei City are to be designed by the end of March 2018. Originally it was intended for the existing V-Category lights to be replaced on a like-for-like basis with new LEDs by June 2018. However, this is no longer possible without compromising the lighting design. This has resulted in the order of these luminaires being delayed until the lighting design is completed in April which, with a 3-4 month lead time, will result in these arriving after 30 June. Discussions with NZTA have confirmed that the 85% FAR will still apply to the luminaire supply for these V-Category lights as long as they are ordered before 30 June.

We are currently determining whether the central management system (CMS) to control the new LED streetlights can be provided by the Spark proposed “Smart Cities” LoRaWAN network.

Kamo Cycleway

The stage 2 contract (Cross St to Kamo Rd) has been awarded and construction has commenced. Construction of the cycleway for Stage 1 were completed by the end of January but works on the Railway and Crossing controls will not be completed until May.

The programme of works and progress is reported below:

Stage 1 (CON16020 Rust Avenue to Cross Street)

- Civil works 99% complete
- Fencing at Whangarei Club on hold. KiwiRail are opting to fix a slump in the upper dry stonewall embankment (near Rust Ave) before we complete this section. Target completion date for all works here is May 2018. This section is to be opened for public use as part of the official opening of the first section (Rust Ave to Manse St).
- Railway signalling to be done by KiwiRail contractor in March-May, with commissioning in mid-May
- Planting will be completed over the next few months.
- Stage 1 has a target official opening of August 2018, but with some sections available to use earlier.

Stage 2 (CON16086 Cross Street to Kamo Road)

- SP 1 Cross Street to Wrack St (target completion of mid-2018)
- Earthworks complete, the section north of Manse Street is nearly ready for pouring.
- Manse St / Lupton Ave intersection improvements is partially completed, next phase is pedestrian signal crossing and rail crossings.
- First section of concrete along Cross Street has been poured.
- SP 2 Wrack St to Kensington Ave has started. Closure of Wrack St intersection with Lupton Avenue is planned for June 2018.
- SP 2 Kensington Ave to Kamo Rd will start mid-2018, target completion date of late-2018.

Stage 3 (Kamo Rd to Jack Street)

- Design completed, awaiting KiwiRail feedback
- Physical works in 2018-19

Stage 4 (Adams Place to Fisher Terrace)

- Path design mostly complete
- underpass preliminary design completed, awaiting KiwiRail 50% review feedback
- Underpass construction likely to take place during the Christmas railway shutdown.

Stage 5 (Fisher Tc to Kamo village)

- Scoping design underway for future links to Kamo Intermediate, Kamo High School and Kamo Village. This will involve a combination of shared paths and traffic calming (greenways).

The Walking and Cycling Strategy

The Walking and Cycling Strategy 2012 is currently undergoing its 5-yearly review. The updated strategy will be the keystone for securing funding for urban walking and cycling projects, as well as strategic regional walking and cycling connections through Whangarei District.

An initial review has been undertaken, which has included consultation with Council's Walking and Cycling Reference Group, Northland Regional Council and other key stakeholders. A workshop with key stakeholders was held on the 9th February. The focus of this workshop was agreeing the major issues and goals for the Strategy, as well as prioritising strategic actions and implementation mechanisms.

It is anticipated that the Draft Strategy will be available for formal public feedback this month.

Whangarei District Road Safety Promotion March 2018

Key district issues
- Young Drivers
- Alcohol and/or drugs
- Speed
- Rural speed zone loss of control / head on
- Intersections

SAiD (Stop Alcohol Impaired Driving): 5 participants completed the programme in March.

Drive Soba: 11 recidivists are currently attending 2 programmes due for completion on 26 April and 2 June respectively. 4 completed the programme which finished on 3 March.

Young Drivers:. Of 40 learners, 34 completed, 32 sat and 30 passed at People Potential 9 out of 10 passed Learners at Ngatiwai Learning Trust.

Speed: A radio advertising campaign is ongoing to target speed around road works. Speed advertising is underway on 1 bus back & 4 cinemas. A second bus back advert commences in April.

Community Mentor Driver Programme: 39 learners are being mentored by 13 volunteers mentors, including 2 new. They carried out 106 hours and 9 sat and 7 passed restricted licences.

Restraints: 2 checking clinics were held at 2 kindergartens, 1 full workshop and Whanau Day at the Falls resulted in 117 interactions, 62 seat checks 22 seats/boosters distributed.

Motorcycle Get Ride Ready Safety Campaign: Some results have been received from ACC but still need to be reconciled with local data. The outcome will be reported next month.

RYDA: 402 students from 7 Secondary Schools in the district attended the Ryda programme for FY18. Possibly two further will attend deliveries on a future date. RRTF supplied all students with water and sausage sizzle for lunch. AMI staff attended with waterproof macpacs had it rained.

Meetings/Other Activities: Meetings: Field Days attendance to promote mentor programme, Northland Road Safety Association, Northland Road Safety Trust, Roadsafe Northland forum, PD: Promoting Sustainability at Wintec

Fatigue Stops:

Date	Vehicles	People	Date	Vehicles	People
20.10.17	69	115	2.02.18	95	220
24.11. 17	28	75	29.03.18	105	208
26 .01.18	95	220	30.04.18		

Northland Safety – Fatigue Driver Stop At Uretiti

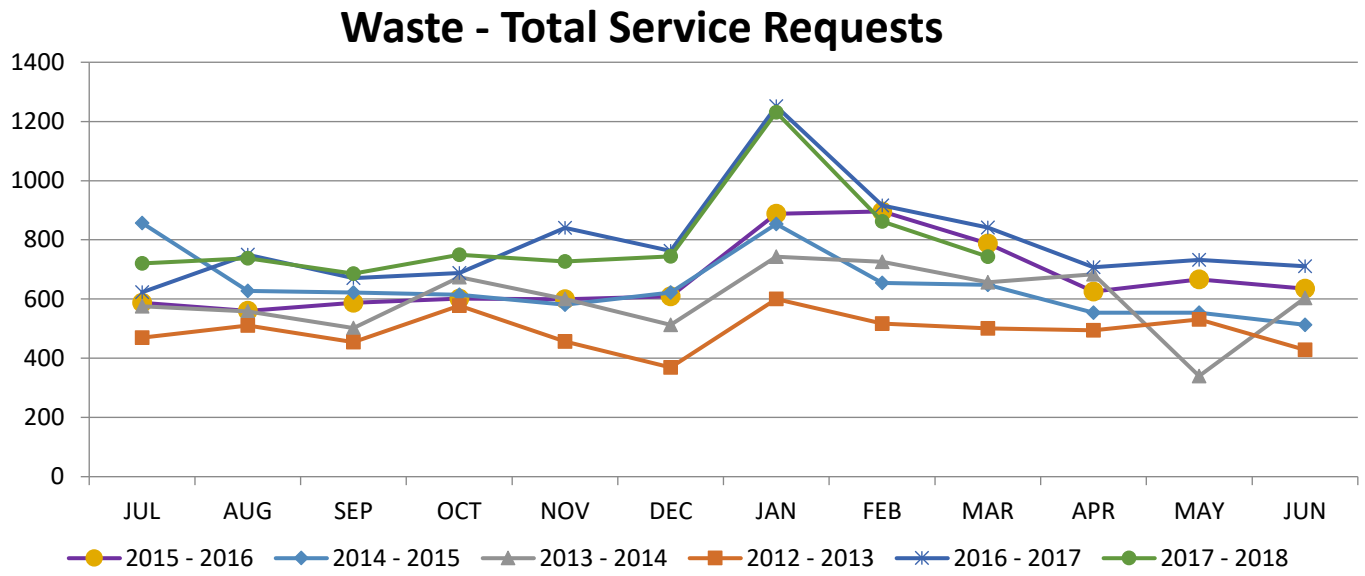
Once again, the volunteers and partners of Northland Safety did a fantastic job at the Fatigue- Driver Reviver stop at Uretiti on the Thursday before Easter. Over 105 vehicles stopped with approximately 208 people enjoyed taking a break during their trip. Many parents with young children loved the focus provided for children at the Stops including appropriate resources. Northland Safety have always recognised the importance to target all occupants of a vehicle especially the young ones as they can cause lots of angst to parents and distractions on long journeys.

Northland Road Toll

Road Toll	Total for all 2016	Total for all 2017	Total at End January 2018	Whangarei District SH Network	Whangarei District Local Network
Whangarei	9	14	8	3	3
Kaipara	9	5	2	1	0
Far North	9	22	5	1	1
Totals	27	41	15	5	4

Customer Request Management Services

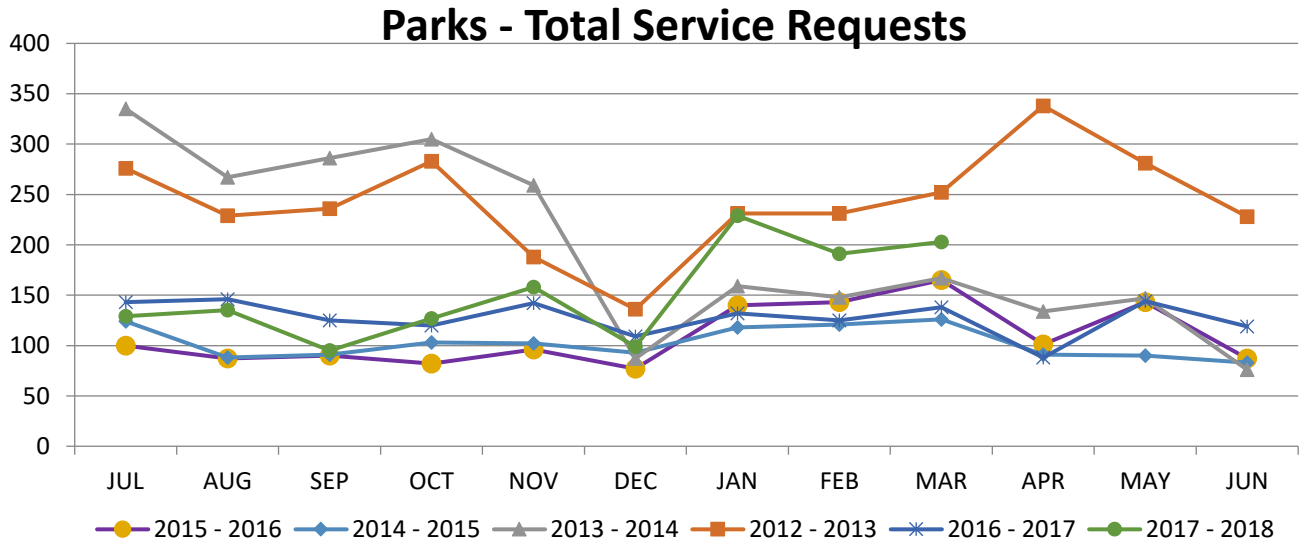
The Infrastructure Group received a total of 1820 CRMs in the month of March 2018. 6,255 CRMs for 2018 to-date with 28,852 CRMs in total for 2017.



The Waste and Drainage Team received 743 CRMs in March 2018. 3 were impressed CRMs and no dissatisfied calls.

The top five CRM issues for our Waste and Drainage Department for the month of March were:

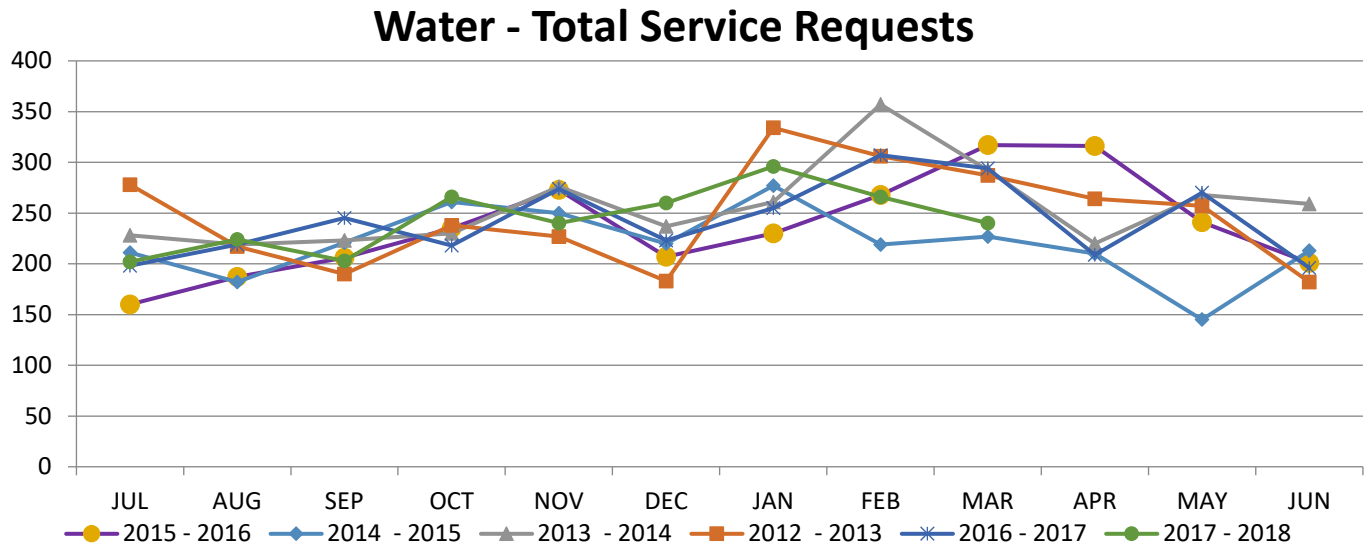
- Rubbish Queries – 127 calls (non-collection, fly tipping etc)
- Public Toilet queries/complaints 58 (eg Soap dispenser empty).
- Recycling queries and complaints- 48 (eg Bin missed during collection)
- Stormwater queries- 31 (eg blocked storm drain)
- Sewer queries- 24 (eg blocked waste drain)



The Parks team received 203 CRMs in March 2018. There were 5 impressed CRM's recorded for the Parks Team in March, along with 1 dissatisfied call.

The top five CRM issues for our Parks and Recreation Department for the month of Marchwere:

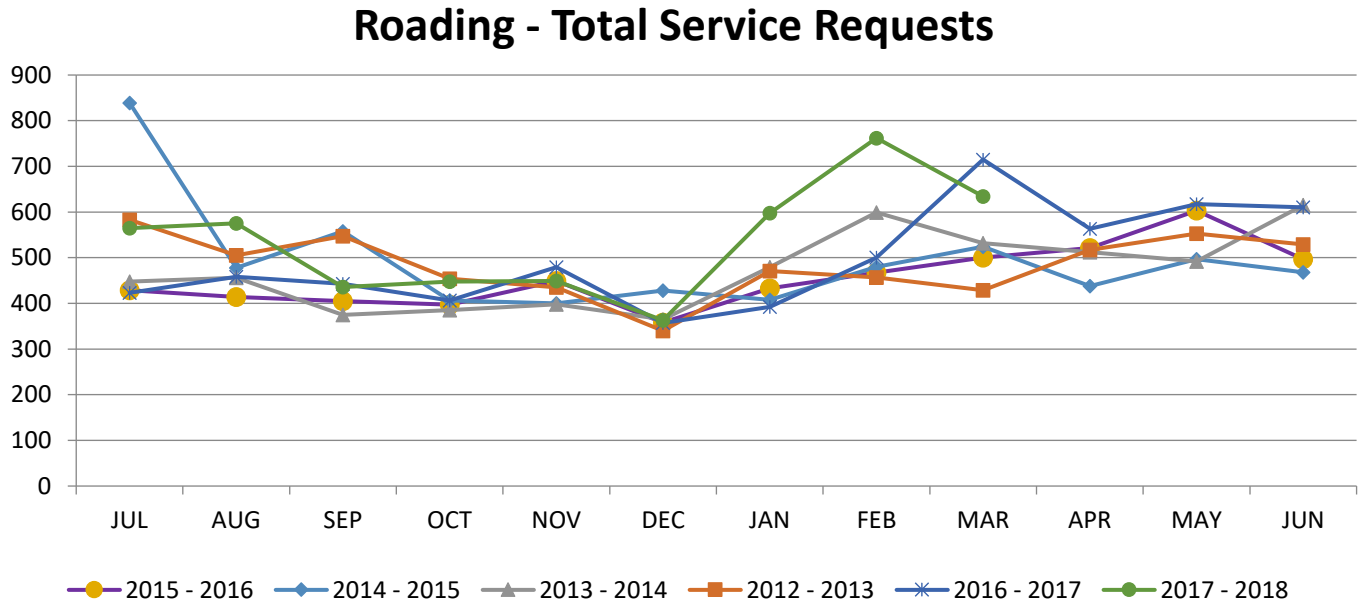
- General Parks queries- 64 (eg access to reserves, Drone requests etc)
- Tree and Street Tree queries- 53 (eg tree fallen over)
- Cemetery enquiries- 15 (Burial enquiries etc)
- Walkways- 15 (Works on walkways)
- Playgrounds 8 (Playground queries and issues)



The Water team received 240 CRMs in March 2018. The Water Team received 11 impressed calls during the month. 2 instances of dissatisfied feedback were received.

The top five CRM issues for our Water Department for the month of March were:

- Water Leaks- 129 (Leak repairs or concerns)
- Meter Box Queries- 32 (New box, new meters)
- General Water Queries- 14 (land enquiries etc)
- Water Quality- 8 (Water quality issues- clarity, odour, taste)
- Water Pressure- 4 (Pressure related queries)



The Roading Team received 634 Customer Service Requests in March 2018. There were 30 follow up calls made in the month of March. 2 customer were dissatisfied. 20 customers found our service acceptable. 8 customers were impressed by the Roading team and contractors.

The top five CRM issues for our Roading Department for the month of March 2018 were:

1.	77 reports of Unsealed Road Issues	E.g. Grading and pot holes
2.	76 reports of Roading General	E.g. General and Safety issues.
3.	73 reports of Parking Meters	E.g. Parking meter issues.
4.	56 reports of Trees and Vegetation	E.g. Maintenance on trees and vegetation affecting Road.
5.	54 reports of Stormwater issues	E.g. Clearing cesspits and stormwater issues.

RESOLUTION TO EXCLUDE THE PUBLIC

That the public be excluded from the following parts of proceedings of this meeting.

The general subject of each matter to be considered while the public is excluded, the reason for passing this resolution in relation to each matter, and the specific grounds under Section 48(1) of the Local Government Official Information and Meetings Act 1987 for the passing of this resolution are as follows:

1.	The making available of information would be likely to unreasonably prejudice the commercial position of persons who are the subject of the information. {Section 7(2)(c)}
2.	To enable the council (the committee) to carry on without prejudice or disadvantage commercial negotiations. {(Section 7(2)(i)}.
3.	To protect the privacy of natural persons. {Section 7(2)(a)}.
4.	Publicity prior to successful prosecution of the individuals named would be contrary to the laws of natural justice and may constitute contempt of court. {Section 48(1)(b)}.
5.	To protect information which is the subject to an obligation of confidence, the publication of such information would be likely to prejudice the supply of information from the same source and it is in the public interest that such information should continue to be supplied. {Section7(2)(c)(i)}.
6.	In order to maintain legal professional privilege. {Section 2(g)}.
7.	To enable the council to carry on without prejudice or disadvantage, negotiations {Section 7(2)(i)}.

Resolution to allow members of the public to remain

If the council/committee wishes members of the public to remain during discussion of confidential items the following additional recommendation will need to be passed:

Move/Second

"That _____ be permitted to remain at this meeting, after the public has been excluded, because of his/her/their knowledge of Item _____.

This knowledge, which will be of assistance in relation to the matter to be discussed, is relevant to that matter because _____.

Note:

Every resolution to exclude the public shall be put at a time when the meeting is open to the public.